## **Geotechnical Evaluation Report**

US Highway 85 Intersection Improvements From Near US 2 and US 85 Junction north to Williams County Road 4 Williams County, North Dakota SOIA-7-085(092)183, PCN 20208

Prepared for

SRF Consulting Group, Inc.

#### **Professional Certification:**

I hereby certify that this plan, specification, or report was prepared by me or und and that I am a duly Regime under the laws of the S

Ezra Ballinger, PE

Associate Principal/Project

Registration Number: PE-732

April 30, 2013

Project FA-12-00359

**Braun Intertec Corporation** 



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April 30, 2013

Project FA-12-00359

Mr. Eric Bach SRF Consulting Group, Inc. Case Plaza, Suite 226 One North 2<sup>nd</sup> Street Fargo, ND 58102

Re: Geotechnical Evaluation Report (Interim Report)

**US Highway 85 Intersection Improvements** 

From Near US 2 and US 85 Junction north to Williams County Road 4

SOIA-7-085(092)183, PCN 20208 Williams County, North Dakota

Dear Mr. Bach:

We are pleased to present this DRAFT Geotechnical Evaluation Report for the proposed construction of a US Highway 85 bypass around the northwest side of Williston, North Dakota in Williams County. Per your request, we are submitting an interim report addressing the southern 2½ miles of the project from the start of the project about ½ mile south of the intersection between US Highway 85 and US Highway 2 north to the intersection between 52<sup>nd</sup> Street NW and 141<sup>st</sup> Avenue NW. A report for the entirety of the project (including the portion discussed herein) will be prepared once drilling has been completed this spring. This project was completed in accordance with our proposal dated February 25, 2013.

In the Appendix of this report we present the Linear Soils Report which summarizes the results of laboratory testing in borings along the proposed route. The Appendix also contains the Boring Logs, Grain Size Accumulation Curves and Proctors. This information is being provided to SRF Consulting Group, Inc. (SRF) and the North Dakota Department of Transportation's (NDDOT) Construction Division, Materials and Research Division, and the Williston District to assist in the roadway design and determination of quantities.

Thank you for making Braun Intertec your geotechnical consultant for this project. If you have questions about this report, or if there are other services that we can provide in support of our work to date, please contact Ezra Ballinger by phone at 701.232.8701 or by email at eballinger@braunintertec.com.

Sincerely,

**BRAUN INTERTEC CORPORATION** 

Ezra Ballinger, PE

Associate Principal/Project Engineer

Steven P. Nagle, PE

Principal Engineer/Vice President



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### **Appendix**

Boring Location Sketches Linear Soils Report Log of Boring Sheets Descriptive Terminology Grain Size Accumulation Curves Proctor Test Results



#### A. Introduction

## A.1. Project Description

The North Dakota Department of Transportation (NDDOT) is planning the construction of a bypass route for US Highway 85 in Williams County around the northwest side of Williston, North Dakota. The selected route will use a combination of existing county or township roads and undeveloped land. The route will proceed northward for about ½ mile from the junction between US Highway 85 and US Highway 2, then northwest to 141<sup>st</sup> Avenue Northwest (NW), north for about 1 mile along 141<sup>st</sup>, then northwest to 142<sup>nd</sup> Avenue NW, north along 142<sup>nd</sup> for about 3 miles, then east along 56<sup>th</sup> Street NW for 1 mile, then northeast to 57<sup>th</sup> Street NW, then east along 57<sup>th</sup> for 4 miles to its junction with US Highway 2 and US Highway 85 north of Williston. This project has been given NDDOT project number SOIA-7-085(092)183, PCN 20208.

The proposed roadway will typically be a four-lane median divided highway. Based on the preliminary plan and profile for the route, the majority of the project will involve cuts or fills of less than 5 feet, with isolated locations, especially in the northern half of the project where grade changes of 10 to 20 feet are planned.

Two new interchanges will also be constructed – one at the north end and one at the south end. In addition a new box culvert will be constructed where 142<sup>nd</sup> Avenue NW crosses Sand Creek just south of 56<sup>th</sup> Street NW.

The scope of our geotechnical evaluation is to provide SRF and the NDDOT with a linear soils survey for the route, an evaluation of the settlement potential under deep fills, recommendations for the new box culvert at Sand Creek and an evaluation of potential borrow sources. It is our understanding that the pavement section will be determined by the NDDOT and that the NDDOT will also be performing the drilling and geotechnical design of the interchanges at the north and south ends of the project.

### A.2. Purpose

The purpose of this geotechnical evaluation is to assist SRF and the NDDOT with the design of the project.



## A.3. Scope of Services

We submitted a proposal to Mr. Eric Bach of SRF on February 25, 2013. Our scope of services in the proposal consisted of the following tasks and subtasks:

- Engineering and Project Management
  - Develop project scope,
  - Site reconnaissance,
  - o Prepare drilling instructions/call in utility locates,
  - Drilling oversight;
  - o Traffic control,
  - Oversee laboratory testing,
  - Prepare boring logs,
  - o Roadway design with regard to the anticipated fills,
  - Attend design meetings/conference calls,
  - o Prepare a draft geotechnical report,
  - o Prepare final geotechnical report, and
  - Overall project management of drilling, laboratory testing, and engineering.

#### Drilling

- Drill 2 standard penetration test borings to an average depth of 40 feet for the box culvert crossing at Sand Creek,
- Drill 140 power auger borings to an average depth of 15 feet at approximately 500 feet spacings along the proposed route,
- o Drill 24 power auger borings to an average depth of 25 feet for borrow areas, and
- Stake boring locations and coordinate with utility companies to locate buried utilities.

#### Laboratory Testing

- Conduct an average of 12 moisture contents, one modified Proctor, one Atterberg limit, and one grain size analysis test for the structure borings,
- Conduct an average of 13 moisture contents, one modified Proctor, one Atterberg limit, and one grain size analysis test for each of the roadway borings, and
- Conduct and average of 13 moisture contents, 1 ½ modified Proctors, 1 ½ Atterberg limits, and 1 ½ grain size analysis tests for each of the borrow borings.

Our scope of work is still ongoing and may be modified as the project progresses.



## B. Results

## **B.1.** Borings

Log of Boring sheets for our test borings are included in the Appendix. The logs identify and describe the geologic materials that were penetrated, and present the results of penetration resistance tests (if any) performed within them, laboratory tests performed on samples retrieved from them, and groundwater measurements. The roadway borings for this project have been numbered sequentially beginning with LSS-01 at the south end of the project and proceeding northward. This interim report addresses borings LSS-01 through LSS-30.

Boring LSS-07 was originally staked and planned to be performed near the center of the existing intersection between US Highway 85 and US Highway 2, however, due to the need for extensive traffic control and safety concerns, as well as buried utilities, that boring was not performed and a suitable offset location was not identified. Boring LSS-09 was originally planned to be advanced about 1000 feet north of the existing US Highway 2 and US Highway 85 intersection on property currently controlled by Cal Frac. However, when we visited the site on separate occasions our drillers were not able to find anyone who would grant access to the property. The majority of our borings were performed within about 12 feet of the proposed roadway centerline except where utilities or access prevented access. The coordinates of the drilled locations are provided on the attached boring logs.

Strata boundaries were inferred from changes in the auger cuttings. In the deeper borings sampling was not performed continuously and the strata boundary depths are only approximate. The boundary depths likely vary away from the boring locations, and the boundaries themselves may also occur as gradual rather than abrupt transitions.

## **B.2.** Geology

A review of geologic information in the vicinity of the site indicates that the soils are glacial till along the majority of the project, and all of the southern 2 ½ miles. Specifically, "Plate 1. Geologic Map of Williams County, North Dakota" (North Dakota Geologic Survey and North Dakota State Water Commission, Bulletin 48, County Groundwater Studies 9, Plate 1, undated) indicates the soils consist primarily of sheet moraine composed of till (an unsorted mixture of gravel, sand, silt, clay and occasional boulders.

For the project, the predominant soil type encountered was A-6 soil.



#### **B.3.** Site Reconnaissance

We have visited the site on numerous occasions during 2012 in conjunction with our Phase I Environmental Site Assessment for the project. We also drove as much of it as we could in order to note the site terrain and evaluate potential drilling issues on the project. The topography along the alignment was typically gently undulating, with some areas near the northern portion of the project where the hills were steeper. For the southern 2 ½ miles that are specifically addressed by this report, the first ½ mile of the project is along the existing Highway 85 alignment south of the intersection with US Highway 2. The next ½ mile proceeds north through Cal-Frac and other industrial property. The proposed route then proceeds northwest for about ¾ mile to the existing County Road 4 alignment where it turns northward along that alignment.

The ditches along US Highway 85 south of Highway 2 were about 2 to 5 feet deep and were covered in snow at the time of our drilling. For the portion of the project that proceeds north and then northwest through the industrial property over to County Road 4, we noted that the roadway alignment appeared to have been rough graded last fall. A few inches of snow was typically present on the ground in the area. A typical view of this portion of the project at the time of our drilling is shown in Figure 1 below. As noted above, we were not able to access the Cal-Frac property immediately north of US Highway 2 because it was gated and blocked off and we were unable to find personnel that would allow us to access the property. Where the proposed alignment follows the existing County Road 4 alignment, we noted that the ditches contained ice, snow, cattails, and were several feet below the roadway centerline in many areas. The ditches, in-slopes and back-slopes were vegetated with native grasses and shrubs.





Figure 1. Site Conditions near Station 9710+00 during drilling



#### **Soil Classification and Comments** B.4.

For the portion of the project addressed by this interim report, we collected a total of 28 bulk samples and 290 moisture content samples from the flight auger. The borings for the linear soils survey were extended to depths of 5 to 20 feet depending on the amount of cut or fill anticipated at the location.

The results of our laboratory testing for the linear soils survey are shown in Tables 1 and 2 below and on the boring logs and laboratory results sheets, and Linear Soils Report which are provided in the Appendix.



Table 1. Summary of Classification, Moisture Content, and Maximum Dry Unit Weight Testing for Roadway Borings

AASHTO			ure Range (%)	In Place Moisture	AASHTO T-180 Optimum Moisture	AASHTO T-180 Maximum Dry Unit
Classifications	Quantity	Min	Max	Average (%)	Average (%)	Weight Average (pcf)
A-1-b	1	5	9	7	5	138
A-2-4	2	13	18	16.1	6	135.5
A-2-6	2	12	19	14.8	6.5	134.5
A-6	19	4	26	11.9	9.2	130.2
A-7-6	4	9	19	14.4	11.0	124.5

Table 2. Summary of Atterberg Limits Testing

AASHTO		Rang		Liquid Limit	Rang	c Limit ge (%)	Plastic Limit	Ra	c Index nge	Plastic Index
Classifications	Quantity	Min	Max	Average (%)	Min	Max	Average (%)	Min	Max	Average
A-1-b	1	NP <sup>1</sup>	NP <sup>1</sup>	NP <sup>1</sup>	NP <sup>1</sup>	$NP^1$				
A-2-4	2	22	23	23	14	14	14	8	9	9
A-2-6	2	24	28	26	13	13	13	11	15	13
A-6	19	25	40	34	10	16	13	13	27	20
A-7-6	4	41	44	43	13	14	14	27	31	29

<sup>1.</sup> NP indicates the sample tested was Non-Plastic.

As can be seen in Table 1, the majority of the soils encountered in the linear soils survey borings were generally 2 to 10 percent wet of their optimum moisture contents as determined by AASHTO T-180. The predominant soils (A-6) were, on the average, about 2 ½ percent over their optimum moisture content. Depending on the time of construction, we anticipate the in place moisture contents are likely to be higher during spring thaw and after heavy rain events.

The soils encountered in those borings are considered moderately to highly frost-susceptible. The soils classified as A-6 and A-7-6 soils are generally considered fair to poor subgrade materials. A group index of 20 or greater indicates very poor subgrade materials. Of the 23 soils that were classified as A-6 or A-7-6 materials, all of them had a group index less than 20 (ranged from 3 to 20) and the average group index was 10.

#### **B.5.** Groundwater

Groundwater was not observed in any of the linear soils survey borings performed to date. The observation periods were relatively short for all of the borings and water can be anticipated in other locations at the time of construction. In addition, seasonal and annual fluctuations in groundwater levels should be anticipated. Elevated water levels should be anticipated following spring thaw and periods of heavy precipitation.

## C. Analyses and Recommendations

## C.1. Proposed Construction

We have been provided with and reviewed preliminary plan and profile drawings for this project prepared by SRF in March 2013. For the portion of the project south of US Highway 2, the existing Highway 85 roadway may be widened and the intersection between Highway 2 and Highway 85 will be modified. Proceeding north from the existing US Highway 2 intersection, the new alignment will be an undivided or median divided highway having 2 to 4 lanes in either direction through the heavy industrial and developed area within about ½ mile north of US Highway 2.

From approximate Station 9735+00 (about ½ mile north of US Highway 2) to the north, the proposed roadway will be a 4 lane, median divided roadway that will also include a frontage road on either side. Several intersections will be created where the roadway will widen to accommodate one or more turning lanes in each direction. Near Station 9770+00 and continuing north to the end of the segment covered by this interim report, the proposed alignment coincides roughly with the existing alignment of County Road 4.

Based on the profile of the roadway we've reviewed, the vertical alignment of the proposed route will generally involve cuts or fills of less than 5 feet except for a couple of isolated areas near Stations 9765+00 to 9785+00, Station 9800+00 and Station 9810+00 where fills of up to about 8 feet at centerline are planned.

The pavement section for the new roadway will be developed by the NDDOT. We understand that all work on the site will be performed in accordance with NDDOT Standard Specifications.

## C.2. Treatment of Organic Soils

Organic soils are present in the ditches for the portions of the alignment along existing roads and also in the entire roadway area where the route will involve entirely new construction. We recommend that all vegetation, root zones and organic topsoils be removed prior to subgrade preparation and placement of new fill in these areas. After the removal of organics, the subgrade should be prepared as indicated in Section C.3. Organic soils that are removed should not be reused as embankment fill; however they could be stockpiled and may be used as dressing on the new embankment slopes.



## C.3. Subgrade Preparation

After vegetation and topsoils have been removed, we recommend 12 inches of subgrade preparation in cut areas, and in fill areas where less than 18 inches of fill will be placed. In fill areas where greater than 18 inches of fill will be placed, it is not necessary to perform subgrade preparation beyond topsoil stripping. Subgrade preparation should comply with NDDOT Specification 230.02 B.2 (Type A).

Compaction control for subgrade preparation should be in accordance with AASHTO T-180 and Specification 203.02G Type A.

If unsuitable soils are present below the topsoil, scarification and drying or overexcavation and replacement of the unsuitable soils could be considered.

## C.4. Subcutting

Based on the conditions encountered in our borings, we do not anticipate that subcutting will be necessary along the majority of the project. Where localized soft spots or unsuitable soils are encountered during subgrade preparation we recommend that subcutting of the soils be performed as necessary and in accordance with NDDOT Specification 203.02 C.

## C.5. Subgrade Drainage

Due to predominantly clay soils with more than 20 percent of their particles passing a #200 sieve, we recommend that drainage be provided for aggregate base placed over the on-site soils. Drainage should be provided by sloping the subgrade and daylighting the aggregate base to the shoulders. Water should not be allowed to infiltrate the clay subgrade but instead flow down the in-slopes and be collected and routed through the ditches and culverts on either side of the road.

#### C.6. Unsuitable Materials

Based on the soils encountered in our borings, we anticipate that the soils encountered in excavations for the project will be suitable for construction of the roadway embankment. As discussed in Section C.2, organic soil deposits should not be used as embankment fill. We recommend that imported soils used as borrow be similar to the existing subgrade soils. Any soils encountered or imported that cannot be moisture conditioned and compacted according to the recommendations of this report should not be used.

#### C.7. Settlement

Settlement will occur due to compression of the soils underlying the new roadway embankments (the foundation soils), as well as settlement of the embankment fill itself.

#### C.7.a. Foundation Soils

Along the majority of the alignment we anticipate foundation soil settlements of less than 1 inch will be realized. The settlements will be greatest where the new fill is the thickest under the roadway and lessen where the amount of fill typically decreases along the in-slope away from the roadway. Where fills of 5 to 8 feet are planned near Stations 9765+00 to 9785+00, Station 9800+00 and Station 9810+00, we anticipate that settlement of about 1 ½ to 2 inches may be realized under the center of the roadway. The majority of the foundation soil settlement along the alignment is likely to occur within a few weeks to two months after construction of the new embankment.

#### C.7.b. Settlement of Embankment Fill

When fill is placed, it will compress under its own weight, resulting in settlement. In clean sand soils, this settlement occurs rapidly (typically during construction), however, with clayey soils, this settlement occurs over many years. We anticipate that the majority of the soils used as embankment fill for this project will be clayey. We estimate that the total settlement of embankment fill (whether sand or clay) will be less than 1 inches for new embankments up to 8 feet thick.

## C.8. Backslopes

Based on experience with similar projects, we have assumed that where necessary the design cross sections will include cutting the existing soil back to a 4H:1V (Horizontal:Vertical) slope outside of the ditches. We understand that the NDDOT would prefer to use 4:1 backslopes wherever possible. If cases exist where it is not possible to use a 4:1, a 3:1 back-slope may be adequate from a stability standpoint, however, site specific evaluations should be performed for any areas under considerations for steepening beyond a 4:1.

We recommend that for any slopes greater than 20 feet tall, benches about 10 feet wide be constructed no more than 20 vertical feet apart to reduce the potential for erosion due to water flowing down the slope face. We also recommend that the back-slopes be planted with native grasses/shrubs, where possible, as a further protection against erosion. We anticipate that excavation can be performed with typical excavation equipment.



#### D. Construction

#### D.1. Excavation

Bedrock that impeded our drilling equipment was not encountered in our borings; therefore it is our opinion that the soils in the borings can be excavated with standard equipment such as scrapers, earth movers and backhoes. Depending on the time of construction, the subgrades may be excessively wet. It may be necessary to limit the activities of rubber-tired equipment directly on the embankment until the soils are dried.

#### D.2. Testing

We recommend density testing of backfill and fill placed for the roadway. As indicated above, we recommend the use of AASHTO T180. The testing frequency should follow NDDOT requirements.

### E. Procedures

### **E.1.** Borings

The linear soils survey borings were drilled with a truck-mounted core and auger drill equipped with power auger. The borings were performed by advancing the auger at 1- or 2-foot intervals and "dead-pulling" the auger to collect moisture content samples off of the auger at 1-foot spacings. A bulk sample of the soil encountered near the anticipated bottom of the aggregate base was collected from the auger after moisture content samples were collected. Sample intervals and corresponding depths are shown on the boring logs.

## E.2. Material Classification and Testing

#### E.2.a. Visual and Manual Classification

The geologic materials encountered were visually and manually classified in accordance with ASTM Standard Practice D 2488. A chart explaining the classification system is attached. Samples were placed in jars or bags and returned to our facility for review and storage.



#### E.2.b. Laboratory Testing

The results of the laboratory tests performed on geologic material samples are noted on or follow the appropriate attached exploration logs. The tests were performed in accordance with AASHTO procedures.

#### E.3. Groundwater Measurements

The drillers checked for groundwater as the borings were advanced, and again after auger withdrawal. The boreholes were then backfilled.

## F. Qualifications

#### F.1. Variations in Subsurface Conditions

#### F.1.a. Material Strata

Our evaluation, analyses and recommendations were developed from a limited amount of site and subsurface information. It is not standard engineering practice to retrieve material samples from exploration locations continuously with depth, and therefore strata boundaries and thicknesses must be inferred to some extent. Strata boundaries may also be gradual transitions, and can be expected to vary in depth, elevation and thickness away from the exploration locations.

Variations in subsurface conditions present between exploration locations may not be revealed until additional exploration work is completed, or construction commences. If any such variations are revealed, our recommendations should be re-evaluated. Such variations could increase construction costs, and a contingency should be provided to accommodate them.

#### F.1.b. Groundwater Levels

Groundwater measurements were made under the conditions reported herein, shown on the exploration logs, and interpreted in the text of this report. It should be noted that the observation periods were relatively short, and groundwater can be expected to fluctuate in response to rainfall, flooding, irrigation, seasonal freezing and thawing, surface drainage modifications and other seasonal and annual factors.



## F.2. Continuity of Professional Responsibility

#### F.2.a. Plan Review

This report is based on a limited amount of information, and a number of assumptions were necessary to help us develop our recommendations. It is recommended that our firm review the geotechnical aspects of the designs and specifications, and evaluate whether the design is as expected, if any design changes have affected the validity of our recommendations, and if our recommendations have been correctly interpreted and implemented in the designs and specifications.

### F.2.b. Construction Observations and Testing

It is recommended that we be retained to perform observations and tests during construction. This will allow correlation of the subsurface conditions encountered during construction with those encountered by the borings, and provide continuity of professional responsibility.

## F.3. Use of Report

This report is for the exclusive use of the parties to which it has been addressed. Without written approval, we assume no responsibility to other parties regarding this report. Our evaluation, analyses and recommendations may not be appropriate for other parties or projects.

#### F.4. Standard of Care

In performing its services, Braun Intertec Corporation used that degree of care and skill ordinarily exercised under similar circumstances by reputable members of its profession currently practicing in the same locality. No warranty, express or implied, is made.



Appendix



11001 Hampshire Avenue So. Minneapolis, MN 55438 PH. (952) 995-2000 FAX (952) 995-2020

Base Dwg Provided By:

Project No: FA1200359

Drawing No: FA1200359

Scale: Drawn By: Date Drawn: Checked By: Last Modified:

Sheet: 1 of 2

## BRAUN INTERTEC

11001 Hampshire Avenue So. Minneapolis, MN 55438 PH. (952) 995-2000 FAX (952) 995-2020

Base Dwg Provided By:

GEOTECHNICAL EVALUATION
WILLISTON NW BYPASS
SEVERAL ROUTES NW OF WILL!

Project No: FA1200359

Drawing No: FA1200359

 Scale:
 1" = 500'

 Drawn By:
 BJB

 Date Drawn:
 4/10/13

 Checked By:
 EB

 Last Modified:
 4/10/13

Sheet: Fig: 2 of 2

PROJECT NO.: FA-12-00359

**PROJECT: Williston NW Bypass** 

Highway 85

Williams County, North Dakota

Braun Intertec Corporation

Phone: (701) 232-8701

PO Box 485, West Fargo, ND



Boring Number		LSS	-001	LSS-002		LS	SS-003	LSS	S-004	LSS	-005
Northing		4295	05.60	429905.18		43	0404.71	4309	904.24	4314	03.73
Easting		11816	607.08	1181623.21		118	31643.38	1181	663.52	11816	883.69
Elevation		0	.0	0.0			0.0	(	0.0	0	.0
Sample Depth		2.1'	- 5'	2' - 5'			2' - 5'	2'	- 5'	2.5	- 5'
% Passing 3/8" Si	eve	10	00	91			89		94	8	6
% Passing No. 4 S	Sieve	9	4	70			69		81	7	0
% Passing No. 10	Sieve	8	7	53			53		68	5	7
% Coarse Sand (-	No. 10, +No. 40)	1	2	9			13		12	1	1
% Fine Sand (-No.	. 40, +No. 200)	2	7	23			23		23	2	9
% Silt (0.075 - 0.00	02 mm)	4	0	17			13		30	1	3
% Clay (<0.002 mr	n)		7	5			5		5	;	5
% Finer than 0.02	mm	1	6	10			10		11	1	1
Frost Group		F	3	F2			F2		F2	F	2
Liquid Limit (-No.	40)	2	8	24			23		28	N	Р
Plastic Limit (-No.	•	1	3	13			14		13	N	IP
Plasticity Index (-I	No. 40)	1	5	11			9		15	N	IP
Soil Color	·	Bro	wn	Brown		[	Brown	Br	own	Bro	own
<b>USCS Classificati</b>	on	S	С	SC			SC	Ç	SC	S	M
Soil Classification	n (AASHTO M-15)	A-6	(3)	A-2-6 (0)		A-	-2-4 (0)	A-2	-6 (1)	A-1-	b (0)
Optimum Moistur	e (%)	7	.0	6.0			6.0		7.0	5	.0
Maximum Dry Der	nsity (pcf)	13	5.0	136.0			134.0	13	33.0	13	8.0
_		2.0	12	2.0	13	2.0	16	2.0	15	2.0	7
		3.0	14	3.0	17	3.0	16	3.0	13	3.0	5
Depth (ft)	Moisture (%)	4.0	10	4.0	19	4.0	16	4.0	12	4.0	9
(top 8 samples)	(top 8 samples)										
A . Maria	D 1. D (1. / . !!)	4		40			10		10		_
Avg. Moisture of S	Sample Depth (all)	1	2	16			16		13		7

PROJECT NO.: FA-12-00359

**PROJECT: Williston NW Bypass** 

Highway 85

Williams County, North Dakota

Braun Intertec Corporation

PO Box 485, West Fargo, ND

Phone: (701) 232-8701



<b>Boring Number</b>		LSS	-006	LSS-008			LSS-010	LS	S-011	LSS	-012
Northing		4319	03.26	432643.84			433640.42	434	139.75	4346	39.32
Easting		11817	703.85	1181734.24		1	181852.83	1181	806.27	11818	378.00
Elevation		0	.0	2087.7			2093.9	20	93.9	209	92.0
Sample Depth		2.2	- 5'	1' - 10'			3' - 10'	0'	- 6'	0' -	10'
% Passing 3/8" Si	eve	7	7	100			100	1	100	10	00
% Passing No. 4 S	Sieve	6	2	98			96		97	S	5
% Passing No. 10	Sieve	5	0	95			93		94	8	8
% Coarse Sand (-I	No. 10, +No. 40)	1	2	12			12		8	1	0
% Fine Sand (-No.	. 40, +No. 200)	2	4	26			26		22	2	<u>'</u> 1
% Silt (0.075 - 0.00	02 mm)	1	1	36			36		43	3	8
% Clay (<0.002 mr	n)	4	4	21			19		22	1	9
% Finer than 0.02	mm	(	9	39			38		39	3	6
Frost Group		F	2	F3			F3		F3	F	3
Liquid Limit (-No.	40)	2	2	31			29		34	] 3	1
Plastic Limit (-No.			4	13			14		15	_	4
Plasticity Index (-I		8		18		15			19		7
Soil Color	•	Bro	own	Brown			Brown	Br	own	Bro	own
<b>USCS Classification</b>	on	S	С	CL			CL		CL	C	;L
Soil Classification	n (AASHTO M-15)	A-2-	4 (0)	A-6 (7)			A-6 (5)	A-6	6 (10)	A-6	5 (6)
Optimum Moisture	e (%)	6	.0	7.0			9.0	(	9.0	9	.0
Maximum Dry Der		13	7.0	135.0			131.0	1;	31.0	13	1.0
	, , ,	2.0	13	1.0	12	3.0	20	0.0	8	0.0	14
		3.0	18	2.0	9	4.0	11	1.0	8	1.0	12
Depth (ft)	Moisture (%)	4.0	18	3.0	9	5.0	9	2.0	13	2.0	13
				4.0	8	6.0	9	3.0	15	3.0	14
				5.0	13	7.0	16	4.0	17	4.0	12
(top 8 samples)	(top 8 samples)			6.0	15	8.0	14	5.0	11	5.0	13
				7.0	15	9.0	14			6.0	14
				8.0	15					7.0	15
Avg. Moisture of S	Sample Depth (all)	1	6	12			13		12	1	4

PROJECT NO.: FA-12-00359

Braun Intertec Corporation

PO Box 485, West Fargo, ND

Phone: (701) 232-8701



PROJECT: Williston NW Bypass
Highway 85

**Williams County, North Dakota** 

Boring Number		LSS	-013	LSS-014		LS	SS-015	LSS	S-016	LSS-	-017
Northing		4351	28.98	435577.16		43	5959.20	4363	324.66	43669	90.07
Easting		11817	'54.64	1181536.53		118	31215.26	1180	874.06	11805	32.97
Elevation		209	1.2	2088.7		2	2083.9	20	79.1	207	4.0
Sample Depth		1' -	10'	1' - 10'		2	2' - 10'	3'	- 10'	1' -	6'
% Passing 3/8" Si	eve	9	9	98			97		95	10	00
% Passing No. 4 S	Sieve	9	7	96			96		93	9	7
% Passing No. 10	Sieve	9	4	93			93		89	9	5
% Coarse Sand (-I	No. 10, +No. 40)	7	7	8			8		7	8	В
% Fine Sand (-No.	40, +No. 200)	2	1	22			22		22	2	1
% Silt (0.075 - 0.00	)2 mm)	3	7	38			39	;	35	3	9
% Clay (<0.002 mr	n)	3	0	25			25		26	95 8 21 39 27 45 F3 40 15 25 Brown CL A-6 (14) 11.0 125.0	7
% Finer than 0.02	mm	4	8	45			45		43	4	5
Frost Group		F	3	F3			F3		F3	F	3
Liquid Limit (-No.	40)	4	2	37			32		39	4	0
Plastic Limit (-No.	40)	1	4	13			12		14	1	5
Plasticity Index (-I	No. 40)	2	8	24			20		25	2	5
Soil Color		Bro	wn	Brown		-	Brown	Br	own	Bro	wn
<b>USCS Classificati</b>	on	С	L	CL			CL	(	CL	С	L
Soil Classification	(AASHTO M-15)	A-7-6	6 (16)	A-6 (12)		ŀ	A-6 (9)	A-6	6 (12)	A-6	(14)
Optimum Moisture	e (%)	10	0.0	9.0			9.0	(	9.0	11	.0
<b>Maximum Dry Der</b>	nsity (pcf)	120	6.0	129.0			133.0	13	31.0	125	5.0
		1.0	12	1.0	12	2.0	8	3.0	16	1.0	12
		2.0	12	2.0	11	3.0	9	4.0	15	2.0	12
Depth (ft)	Moisture (%)	3.0	10	3.0	16	4.0	12	5.0	11	3.0	15
		4.0	11	4.0	16	5.0	14	6.0	12	4.0	15
		5.0	14	5.0	7	6.0	14	7.0	14	5.0	15
(top 8 samples)	(top 8 samples)	6.0	14	6.0	15	7.0	14	8.0	14		
		7.0	14	7.0	15	8.0	14	9.0	14		
		8.0	12	8.0	16	9.0	15				
Avg. Moisture of S	Sample Depth (all)	1	3	14			13		14	1.	4

PROJECT NO.: FA-12-00359

**PROJECT: Williston NW Bypass** 

Highway 85

Williams County, North Dakota

Braun Intertec Corporation

Phone: (701) 232-8701

PO Box 485, West Fargo, ND



Boring Number		LSS	-018	LSS-019		LS	SS-020	LSS	S-021	LSS	-022
Northing		4370	55.53	437434.57		43	7881.39	4383	372.07	4388	71.65
Easting		11801	91.80	1179866.72		117	9646.29	1179	559.86	11795	75.02
Elevation		207	74.0	2074.2		2	2077.0	20	88.2	209	94.5
Sample Depth		1' -	10'	1' - 10'			1' - 6'	0.5	5' - 6'	3' -	15'
% Passing 3/8" Si	ieve	9	6	90			99	1	00	9	9
% Passing No. 4 S	Sieve	9	4	87			99	!	97	9	7
% Passing No. 10	Sieve	9	0	82			97		92	9	2
% Coarse Sand (-	No. 10, +No. 40)		0	7			7		13	(	9
% Fine Sand (-No	. 40, +No. 200)	2	2	21			16		25	2	0
% Silt (0.075 - 0.00	02 mm)	3	5	32			43	;	35	3	5
% Clay (<0.002 mi	m)	2	4	23			30		20	2	8
% Finer than 0.02	mm	3	9	37			52		38	4	6
Frost Group		F	3	F3			F3		F3	F	3
Liquid Limit (-No.	40)	3	6	34			38		25	3	9
Plastic Limit (-No.		1	4	14			16		10	1	2
Plasticity Index (-		2	2	20			22		15	2	7
Soil Color	,	Bro	own	Brown		Е	Brown	Br	own	Bro	own
<b>USCS Classificati</b>	on	C	;L	CL			CL	(	CL	C	:L
Soil Classification	n (AASHTO M-15)	A-6	(9)	A-6 (7)		Α	-6 (14)	A-	6 (5)	A-6	(14)
Optimum Moistur	e (%)	9	.0	10.0			10.0	(	9.0	10	0.0
Maximum Dry Dei	nsity (pcf)	13	1.0	130.0			124.0	13	32.0	12	9.0
-		1.0	11	1.0	14	1.0	14	1.0	8	3.0	10
		2.0	9	2.0	17	2.0	9	2.0	5	4.0	10
Depth (ft)	Moisture (%)	3.0	14	3.0	26	3.0	7	3.0	4	5.0	11
		4.0	14	4.0	19	4.0	8	4.0	4	6.0	11
		5.0	14	5.0	15	5.0	9	5.0	6	7.0	10
(top 8 samples)	(top 8 samples)	6.0	14	6.0	14					8.0	10
		7.0	15	7.0	13					9.0	10
		8.0	15	8.0	16					10.0	20
Avg. Moisture of S	Sample Depth (all)	1	4	17			9		5	1	2

PROJECT NO.: FA-12-00359

**PROJECT: Williston NW Bypass** 

Highway 85

Williams County, North Dakota

Braun Intertec Corporation

PO Box 485, West Fargo, ND

Phone: (701) 232-8701



<b>Boring Number</b>		LSS	-023	LSS-024		LS	SS-025	LSS	S-026	LSS-	027
Northing		4393	71.36	439871.12		44	0370.16	4408	869.92	44136	9.55
Easting		11795	88.63	1179576.81		117	9547.45	1179	539.35	11795	55.55
Elevation		210	1.6	2105.4		2	2100.9	20	92.1	208	7.7
Sample Depth		0.5'	- 15'	2' - 7'		0.	5' - 10'	0.5	' - 10'	0.3'	- 5'
% Passing 3/8" Si	eve	9	8	98			99		99	99	9
% Passing No. 4 S	Sieve	9	6	95			96		97	9	5
% Passing No. 10	Sieve	94		89			91	93		90	)
% Coarse Sand (-I	No. 10, +No. 40)	6	3	14			8		CL         CL           A-6 (8)         A-6 (10)	)	
% Fine Sand (-No.	. 40, +No. 200)	19		27			19		23	22	2
% Silt (0.075 - 0.00	02 mm)	3	4	31			35		34	38	3
% Clay (<0.002 mr	n)	3	5	17			29		27	2	1
% Finer than 0.02	mm	5	3	31			47	,	44	39	)
Frost Group		F	3	F3			F3		F3		
Liquid Limit (-No.	40)	4	4	26			40		30	3	7
Plastic Limit (-No.	40)	1	3	13			13		11	16	3
Plasticity Index (-I	No. 40)	3	1	13			27		19	2	1
Soil Color	•	Bro	wn	Brown		Brown		Br	own	Dark E	Brown
<b>USCS Classificati</b>	on	С	L	SC			CL		CL	С	L
Soil Classification	n (AASHTO M-15)	A-7-6	5 (19)	13         27         19         21           Brown         Brown         Brown         Dark Bro           SC         CL         CL         CL           9)         A-6 (3)         A-6 (14)         A-6 (8)         A-6 (10)		(10)					
Optimum Moistur	e (%)	11	.0	8.0			10.0		9.0	11	.0
Maximum Dry Der	nsity (pcf)	12	7.0	137.0			129.0	1:	32.0	Dark Brown CL A-6 (10)  11.0 123.0 1.0 1	3.0
		1.0	16	2.0	12	1.0	13	1.0	9	1.0	10
		2.0	9	3.0	9	2.0	12	2.0	6	2.0	9
Depth (ft)	Moisture (%)	3.0	9	4.0	10	3.0	11	3.0	7	3.0	8
		4.0	10	5.0	9	4.0	11	4.0	9	4.0	5
		5.0	9	6.0	8	5.0	11	5.0	9		
(top 8 samples)	(top 8 samples)	6.0	10			6.0	10	6.0	10		
		7.0	10			7.0	11	7.0	11		
		8.0	11			8.0	12	8.0	12		
Avg. Moisture of S	Sample Depth (all)	1	1	10			11		9	8	

PROJECT NO.: FA-12-00359

**PROJECT: Williston NW Bypass** 

Highway 85

**Williams County, North Dakota** 

Braun Intertec Corporation
PO Box 485, West Fargo, ND
INTERTEC

Phone: (701) 232-8701

<b>Boring Number</b>		LSS	-028	LSS-029		LS	SS-030	
Northing		4418	69.08	442368.53		44	2867.99	
Easting		11795	76.65	1179597.71		117	9618.93	
Elevation		210	06.2	2112.2		2	2117.9	
Sample Depth		0.3'	- 10'	0.3' - 15'		0	.3' - 5'	
% Passing 3/8" Si	eve	9	9	100			99	
% Passing No. 4 S	Sieve	9	6	99		98		
% Passing No. 10	Sieve	9	3	96			95	
% Coarse Sand (-I	No. 10, +No. 40)		7	7		8		
% Fine Sand (-No.	40, +No. 200)	1	9	20		26		
% Silt (0.075 - 0.00	)2 mm)	3	9	40	40		36	
% Clay (<0.002 mr	n)	2	8	30		24		
% Finer than 0.02	mm	4	.9	50			41	
Frost Group	st Group							
Liquid Limit (-No.	40)	4	4	41		34		
Plastic Limit (-No.	•	1	4	14		13		
Plasticity Index (-I		3	0	27		21		
Soil Color	•	Bro	own	Brown		Е	Brown	
<b>USCS Classification</b>	on	С	:L	CL		CL		
Soil Classification	n (AASHTO M-15)	A-7-6	6 (17)	A-7-6 (16)		A-6 (9)		
Optimum Moisture	e (%)	12	2.0	11.0			9.0	
Maximum Dry Der	nsity (pcf)	12:	2.0	123.0			126.0	
		1.0	19	1.0	17	1.0	6	
		2.0	19	2.0	17	2.0	6	
Depth (ft)	Moisture (%)	3.0	19	3.0	17	3.0	6	
		4.0	18	4.0	17	4.0	9	
		5.0	18	5.0	17			
(top 8 samples)	(top 8 samples)	6.0	18	6.0	17			
		7.0	18	7.0	18			
		8.0	18	8.0	18			
Avg. Moisture of S	Sample Depth (all)	1	8	17			7	

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		t FA-12-00	0359	BORING	:		LS	S-001		
Willist Severa					LOCATIO E118160	ON: Sta 7.083 S	ation See SI	9676 ketch.	+00; N42950	)5.595,
DRILLE			METHOD:	Power Auger	DATE:	3/2	5/13		SCALE:	1'' = 4'
Depth feet 0.0	ASTM Symbol			ption of Materials  I D2488 or D2487)		BPF	WL	MC %	Tests	or Notes
0.6	BIT AGG	FILL: Claye A-6 (3) MDD = 135. END OF BO Bag sample of	Bituminous Surf Aggregate Base y Sand, trace G 0 pcf; OMC = PRING. collected from 2 pserved with 5 fe	ravel, brown, moist.	ad			3 10 12 14 10	LL=28, PL=13, P	I=15; P200=48%
-										



	n Projec		00359		<u> </u>	BORING				S-002	
Willis Sever	ton NW E al Routes,					LOCATIO E1181623	ON: St. 3.211 S	ation See S	9680 ketch	+00; N4299	05.182,
DRILL			METHOD:	Power Auger		DATE:	3/2	5/13		SCALE:	1" = 4
Depth feet 0.0		61/8:1	(ASTM	iption of Materials I D2488 or D2487)			BPF	WL	MC %	Tests	or Notes
- 0.5 - 2.0	AGG	18 inches	es of Bituminous of Aggregate Base	e.					4 5		
_	FILL	A-2-6 (0)	yey Sand with Gra 36.0 pcf; OMC =	evel, brown, moist. = 6.0%.		_			13 17	LL=24, PL=13, 1	PI=11; P200=2
5.0		END OF I	BORING.						19		
_			le collected from 2			-					
_			observed with 5 fen backfilled.	eet of power auger in	the ground	l. –					
_						_					
_						_					
_						_					
_						_					
_						_					
_						_					
_						_					
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											SS-002 page



		t FA-12-00	)359		]	BORING			LS	S-003	
Willist Severa				]	LOCATIO E1181643	ON: Sta 3.377 S	ation See Sl	9685 ketch.	+00; N43040	)4.711,	
DRILLI	ER: S. We	enko	METHOD:	Power Auger	]	DATE:	3/2	5/13		SCALE:	1" = 4'
Depth feet 0.0	ASTM Symbol			ption of Materials D2488 or D2487)			BPF	WL	MC %	Tests	or Notes
5.0	_	FILL: Claye A-2-4 (0) MDD = 134.  END OF BO Bag sample of	Bituminous Surf Aggregate Base y Sand with Gra 0 pcf; OMC = PRING. collected from 2 pserved with 5 fe	acing. e. avel, brown, moist. = 6.0%.	n the ground				3 6 16 16 16	LL=23, PL=14, P	I=9; P200=17%
						- - - -					



		t FA-12-00359	BORING	ł:		LS	S-004			
Willist Severa			LOCATI E118166	ON: St 3.52 Se	ON: Station 9690+00; N430904.24, 3.52 See Sketch.					
DRILLI	ER: S. W	enko METHOD: Power Auger	DATE:	3/2	5/13		SCALE:	1'' = 4'		
Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes		
0.5	BIT	6 1/2 inches of Bituminous Surfacing.				4				
2.0	AGG	17 inches of Aggregate Base.	-			3				
2.0	FILL X	FILL: Clayey Sand with Gravel, brown, moist.	_	T)		15	LL=28, PL=13, P	I=15; P200=34%		
-		A-2-6 (1) MDD = 133.0 pcf; OMC = 7.0%.	_			13				
			-			12				
5.0		END OF BORING.		1						
		Bag sample collected from 2 to 5 feet.	-	-						
		Water not observed with 5 feet of power auger in t	he ground	-						
		•	- Broana.	-						
		Boring then backfilled.	-	-						
				1						
			-	41						
			_							
			_							
			_							
-			_							
•			_							
			-	1						
			_	-						
				-						
			-	-						
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•			_	1						
•			_	1						
				1						
			-	-						



		YA-12-00359	BORING	BORING: LSS-005							
	W Bypa utes, NW		LOCATIO E1181683	ON: St 3.686 S	ation See S	9695 ketch	5+00; N43140	00; N431403.733,			
DRILLER:	S. Wenko	METHOD: Power Auger	DATE:	3/2	5/13		SCALE:	1'' = 4'			
Depth feet AST 0.0 Sym	ıbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes			
0.0 Sym	FII mc A-MI Ba	(ASTM D2488 or D2487)  1/2 inches of Bituminous Surfacing.  1 inches of Aggregate Base.  LL: Silty Sand with Gravel, fine- to coarse-grain oist.  1-1-b (0)  IDD = 138.0 pcf; OMC = 5.0%.  ND OF BORING.  ag sample collected from 2.5 to 5 feet.  Vater not observed with 5 feet of power auger in the poring then backfilled.	_ _ _			2 2 7 5 9	LL=NP, PL=NP,	PI=NP; P200=18			



			t FA-12-	しいいこうグ		L	BORING							
Willis Sever	ton N al Ro	NW B outes,	valuation ypass NW of Wi y, North D				LOCATIO E1181703	ON: St 3.853 S	ation See S	9700 ketch	+00; N4319	03.262,		
DRILL	ER:	S. We	enko	METHOD:			DATE:	3/2	5/13		SCALE:	1'' = 4		
Depth feet 0.0	Syr	TM nbol		(ASTN	ription of Materials M D2488 or D2487)			BPF	WL	MC %	Tests	or Notes		
	Syr BIT AGC	nbol	20 inches  FILL: Cla A-2-4 (0)  MDD = 13  END OF I  Bag sampl	(ASTM es of Bituminous of Aggregate Base ayey Sand with Grand Strategy Sand with Grand Strategy Sand	M D2488 or D2487) Surfacing. se. ravel, brown, moist. = 6.0%.	n the groun	d				LL=22, PL=14, H			
- - - - - - - -					Braun Intertec C		- - - - -					S-006 pag		



		ect FA-1		BORING: LSS-008					
Willist Severa	ton NW al Route	Evaluation Bypass s, NW of V nty, North	Williston	LOCATIO E1181734	ON: St 4.235 S	ation See S	9705 ketch	+00; N43264	13.844,
DRILLI	ER: J. I	Brooks	METHOD: Power Auger	DATE:	3/1:	2/13		SCALE:	1'' = 4'
Elev. feet 2087.7	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes
2086.7	1.0	FILL	FILL: Poorly Graded Sand, fine- to coarse-grain Gravel and Scoria, wet.  SANDY LEAN CLAY, trace Gravel, brown, model A-6 (7)  MDD = 135.0 pcf; OMC = 7.0%.	,			18 12 9 9 8 13 15	LL=31, PL=13, Pl	=18; P200=57%
- 2077.7 - -	10.0		END OF BORING.  Bag sample collected from 1 to 10 feet.  Water not observed with 10 feet of power auger ground.	in the			15 15 15		
- - - -			Boring then backfilled.	- - - -					
·				- - - -					
· ·				- - - -					



	n Proje chnical l		-12-00359	BORING				SS-010	
Willis Severa	ton NW al Route	Bypass s, NW		E118185	ON: St 2.831 S	ation See S	9715 ketch	5+00; N433640.419,	
DRILLI		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE: 1" = 4	
Elev. feet 2093.9	Depth feet 0.0		(ASTM D2488 or D2487)					Tests or Notes	
2092.9	1.0	FILL FILL	FILL: Poorly Graded Sand, with Gravel, fine-t coarse-grained, brown and gray, moist.  FILL: Sandy Lean Clay, with Gravel, brown, m				6 14		
2090.9	3.0	CL	SANDY LEAN CLAY, trace Gravel, brown, m A-6 (5)	oist.			14 20	LL=29, PL=14, PI=15; P200=5	
			MDD = 131.0 pcf; OMC = 9.0%.				9		
_				-			9 16		
2083.9	10.0						14		
_			END OF BORING.  Bag sample collected from 3 to 10 feet.	_					
_			Water not observed with 10 feet of power auger ground.	in the	-				
_			Boring then backfilled.	_	-				
-				-	-				
-				-	-				
					-				
_				-					
-				_	-				
_				-	_				
_				-					
					-				
				_					



Geotecl	hnical l	Evaluati	12-00359 on	BORING LOCATI	ON: St	ation	9720	<b>S-011</b> +00; N4341	39.745
	Route	s, NW of	Williston h Dakota	E118180	6.274 \$	See S	ketch	-	
DRILLE		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1":
Elev. feet 2093.9	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Not
2093.9	6.0	CL	FILL: Sandy Lean Clay, a little Gravel, brown A-6 (10) MDD = 131.0 pcf; OMC = 9.0%.  SANDY LEAN CLAY, a little Gravel, brown,  END OF BORING.  Bag sample collected from 0 to 6 feet.  Water not observed with 10 feet of power augground.  Boring then backfilled.	moist.			8 8 13 15 17 11 13 11 15 15	LL=34, PL=15, F	T=19; P20
- - - - - - - - - - -			Braun Intertec Corporation	- - - - - -					S-011



Geote	chnical 1	Evaluatio	2-00359 n	BORING		ation		S-012 +00; N4346	39 324
Severa		s, NW of	Williston h Dakota	E118187	7.998 S	See S	ketch		39.324,
DRILLI		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1"=
Elev. feet 2092.0	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)			WL	MC %	Tests	or Note
_		FILL	FILL: Sandy Lean Clay, a little Gravel, brov	wn, moist.			14 12	LL=31, PL=14 P	I=17; P200=
-			MDD = 131.0  pcf; OMC = 9.0%.	-			13		
2088.5	3.5	CL	SANDY LEAN CLAY, a little Gravel, brow	/n, moist			14		
							12		
_				-			14		
_				_			15		
_				_			15		
2082.0	10.0		END OF BORING.						
_			Bag sample collected from 0 to 10 feet.	_					
_			Water not observed with 10 feet of power at ground.	ager in the					
-			Boring then backfilled.	_					
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-				-					
A-12-0035			Braun Intertee Corpora						S-012 pa



Geotec	hnical l	Evalua	ation	2-00359	BORING LOCATION	ON: St	ation	9730	SS-013 0+00; N4351	28.978,
Severa		s, NW	of V	Villiston Dakota	E1181754	4.635 S	See S	ketch		,
DRILLE		Brooks		METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1" =
Elev. feet 2091.2	Depth feet 0.0	AST Syml		Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Note
2090.2	1.0	CL		LEAN CLAY with SAND, trace Gravel, brown,	wet.			18		
	1.0	CL		SANDY LEAN CLAY, trace Gravel, brown, modamp. A-7-6 (16) MDD = 126.0 pcf; OMC = 10.0%.	ist to			12 12 10 11 14 14	LL=42, PL=14, 1	PI=28; P200
_					_			14		
-					_			12		
2081.2	10.0							15		
				END OF BORING.	_					
_				Bag sample collected from 1 to 10 feet.	_					
_				Water not observed with 10 feet of power auger ground.	in the _					
_				Boring then backfilled.	_					
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  -										
FA-12-0035	2			Braun Intertec Corporation						S-013 p



	n Proje chnical l		-12-00359 ion	BORING		otics		S-014 +00; N4355	77 14
Severa		s, NW (	f Williston rth Dakota	E1181530	5.53 Se	ee Sk	etch.	+00, N4333	//.10,
DRILLE		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1"
Elev. feet 2088.7	Depth feet 0.0	ASTM Symbo			BPF	WL	MC %	Tests	or No
2087.7	1.0	CL	SANDY LEAN CLAY, trace Gravel, brown, v	wet.			20		
	1.0	CL	SANDY LEAN CLAY, a little Gravel, brown, moist. A-6 (12) MDD = 129.0 pcf; OMC = 9.0%.	damp to			12 11 16 16 7	LL=37, PL=13, 1	PI=24; P2
_				-			15 15		
-	10.0			_			16 17		
2078.7	10.0		END OF BORING.						
_			Bag sample collected from 1 to 10 feet.	_					
-			Water not observed with 10 feet of power aug ground.	er in the					
_			Boring then backfilled.						
_				_					
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	n Proje chnical l		-12-00359	BORING				S-015	
Willis Severa	ton NW al Route	Bypass s, NW o	f Williston rth Dakota	LOCATION E118121:	ON: St 5.262 S	ation See S	9740 ketch	)+00; N4359	59.198,
DRILL		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1" = 4
Elev. feet 2083.9	Depth feet 0.0	ASTM Symbo		•	BPF	WL	MC %	Tests	or Notes
- 2081.9 -	2.0	FILL	FILL: Sandy Lean Clay, trace Gravel, brown,  SANDY LEAN CLAY, trace Gravel, brown,  A-6 (9)  MDD = 133.0 pcf; OMC = 9.0%.	_			13 10 8 9	LL=32, PL=12, I	PI=20; P200=63
-				- - -			12 14 14		
2073.9	10.0			-			14 14 15		
	10.0		END OF BORING.  Bag sample collected from 2 to 10 feet.	_	-				
_			Water not observed with 10 feet of power aug ground.	er in the					
_			Boring then backfilled.	_	-				
_				_	-				
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A-12-0035	59		Braun Intertec Corporation	on .	Ш			IS	S-015 page



		ect FA-1 Evaluatio	2-00359	BORING		··		SS-016	(50
Willis Severa	ton NW al Route	Bypass s, NW of	Williston h Dakota	E118087	ON: St 4.062 S	ation See S	9745 ketch	5+00; N436324	.658,
DRILL		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1" = 4
Elev. feet 2079.1	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests or	Notes
_		FILL	FILL: Sandy Lean Clay, trace Gravel, brown, n -Organic, dark brown and black below 1 foot.	noist. - -			13		
2076.1	3.0	CL	SANDY LEAN CLAY, a little Gravel, brown, r A-6 (12)	noist.			18 16	LL=39, PL=14, PI=2	25; P200=6
			MDD = 131.0  pcf; OMC = 9.0%.				15		
_				-			12		
2069.1	10.0			_			14		
_			END OF BORING.  Bag sample collected from 3 to 10 feet.	_	-				
_			Water not observed with 10 feet of power auger ground.	in the	-				
			Boring then backfilled.	_	-				
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A-12-0035	50		Braun Intertee Corporation					1.00	)16 page



			2-00359	BORING			LS	S-017
Willist Severa	ton NW al Route	Evaluation Bypass s, NW of Yorth nty, North	Williston	LOCATIO E1180532	ON: St 2.972 S	ation See S	9750 ketch	+00; N436690.071,
DRILLE		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE: 1" = 4"
Elev. feet 2074.0	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests or Notes
2073.0	1.0	CL ////	SANDY LEAN CLAY, a little Gravel, brown	moist.			16	
2073.0	1.0	CL	SANDY LEAN CLAY, trace Gravel, brown, 1	noist.			12	LL=40, PL=15, PI=25; P200=669
			A-6 (14) MDD = 125.0 pcf; OMC = 11.0%.	_			12	
				_			15	
2068.0	6.0						15	
	3.0	CL	LEAN CLAY with SAND, trace Gravel, gray,	moist.			19	
				_			17	
				_			15	
2064.0	10.0		FND OF BODDIG				14	
			END OF BORING.	_				
			Bag sample collected from 1 to 6 feet.	_				
			Water not observed with 10 feet of power aug ground.	er in the				
-			Boring then backfilled.	_				
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Geotec				-00359	BORING		otion		<b>S-018</b> +00; N4370	55 522
Willisto	on NW Routes	Bypa s, NW	ss / of W	Villiston	E118019	JN: St 1.799 S	See S	ketch	+00; N43/0.	33.333,
DRILLE		Brooks		METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1'' =
Elev. feet 2074.0	Depth feet 0.0	AST Sym		Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Note
2073.0	1.0	CL		SANDY LEAN CLAY, with Gravel, trace org brown, moist.	ganics,			13		
		CL		SANDY LEAN CLAY, a little Gravel, brown	, moist.			11	LL=36, PL=14, F	PI=22; P200
				A-6 (9) MDD = 131.0 pcf; OMC = 9.0%.	_			9		
_					_			14		
								14		
_					_			14		
_					_			14		
					_			15		
					_			15		
2064.0	10.0			END OF DODING		1		18		
				END OF BORING.	_	-				
_				Bag sample collected from 1 to 10 feet.	_	-				
2064.0				Water not observed with 10 feet of power aug ground.	er in the	_				
				Boring then backfilled.	_	-				
						-				
-					_	1				
-					_	-				
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						1				
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FA-12-00359				Braun Intertec Corporation						S-018 p



COCATION: Station 9760+000 N437434-568   E1179866.717   See Sketch.				12-00359	BORING				S-019	
DRILLER: J. Brooks   METHOD: Power Auger   DATE: 3/12/13   SCALE: 1"	Willist Severa	on NW I Routes	Bypass s, NW o	f Williston	LOCATION E117986	ON: St 6.717 S	ation See S	9760 ketch	)+00; N4374	34.568,
Feet   Geet   ASTM   Description of Materials   BPF   WL   MC   MC   MC   MC   MC   MC   MC   M					DATE:	3/1	2/13		SCALE:	1" = 4
2073.2 1.0 moist.  CL SANDY LEAN CLAY, a little Gravel, brown, moist. A-6 (7) MDD = 130.0 pcf; OMC = 10.0%.  -wet at 3 feet.  14 12 15 14 13 16 16 16 16  END OF BORING. Bag sample collected from 1 to 10 feet. Water not observed with 10 feet of power auger in the ground.	feet	feet		(ASTM D2488 or D2487)		BPF	WL		Tests	or Notes
	2073.2	1.0	CL	SANDY LEAN CLAY, trace Gravel and organ moist.  SANDY LEAN CLAY, a little Gravel, brown, A-6 (7) MDD = 130.0 pcf; OMC = 10.0%.  -wet at 3 feet.  END OF BORING.  Bag sample collected from 1 to 10 feet.  Water not observed with 10 feet of power auge ground.	moist			12 14 17 26 19 15 14 13	LL=34, PL=14, I	PI=20; P200=:
FA-12-00359 Braun Intertec Corporation LSS-019						-				



		ect FA-1 Evaluatio	2-00359 n	BORING		-4°:		S-020	01 201
Willist Severa	ton NW al Route	Bypass s, NW of	Williston h Dakota	E117964	ON: St 6.293 S	ation See S	9/65 ketch	+00; N4378	81.391,
DRILLI		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1" =
Elev. feet 2077.0	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes
2076.0	1.0	FILL	FILL: Lean Clay with Sand, trace Gravel, roots a Organics, brown, wet.	nd			16		
	6.0	CL	LEAN CLAY with SAND, trace Gravel, brown, r damp. A-6 (14) MDD = 124.0 pcf; OMC = 10.0%.	- - -			14 9 7 8 9	LL=38, PL=16, I	PI=22; P200=
		CL	SANDY LEAN CLAY, a little Gravel, brown, da	.mp. - -			9 8 8		
							10 9 11		
- - -				-			11 11 11		
2062.0	15.0		END OF BORING.		<u> </u>				
_			Bag sample collected from 1 to 6 feet.	=	1				
_			Water not observed with 15 feet of power auger is ground.	n the	-				
_			Boring then backfilled.		-				
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_ _				-	-				
_									
_ FA-12-0035	0		Braun Intertee Corporation	_	-			10	S-020 pa



Geotec	hnical l	Evaluati	12-00359 on	BORING		ation		<b>SS-021</b> 0+00; N4383	72.068
Severa		s, NW o	f Williston th Dakota	E117955	9.86 Se	ee Sk	etch.	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	2.000,
DRILLE		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1" =
Elev. feet 2088.2	Depth feet 0.0	ASTM Symbol			BPF	WL	MC %	Tests	or Notes
2087.7	0.5	CL CL	SANDY LEAN CLAY, with Gravel, roots and brown and black, moist.  (Topsoil)  SANDY LEAN CLAY, trace Gravel, brown, d A-6 (5)  MDD = 132.0 pcf; OMC = 9.0%.	-			15 8 5 4 4	LL=25, PL=10, P	I=15; P200=
2082.2	6.0	CL	SANDY LEAN CLAY, a little Gravel, brown a gray, moist.	and light			6 12		
_				-			11 13 12		
_				<u>-</u> - -			15 14 14		
2073.2	15.0		END OF BORING.	-			13 14		
_			Bag sample collected from 0.5 to 6 feet.  Water not observed with 15 feet of power auge	r in the	_				
_			ground.  Boring then backfilled.	- -	-				
_				-	_				
- - 				- -	-				
_				-	-				
_				- -	-				
FA-12-00359	)		Braun Intertec Corporation	1	-			LS	S-021 pa



Geotec	hnical l	Evalu	ıation	2-00359	BORING	ON: St	ation	9775	<b>S-022</b> +00; N4388	71.648,
Severa		s, ÑV	V of V	Villiston Dakota	E117957	5.021 \$	See S	ketch		
DRILLE		Brooks			DATE:	3/1	2/13		SCALE:	1" = 4
Elev. feet 2094.5	Depth feet 0.0	AS'		Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes
_		СН		FAT CLAY, a little Gravel, brown, wet.	-			28 26 25		
2091.5	3.0	CL		SANDY LEAN CLAY, trace Gravel, brown, mois A-6 (14) MDD = 129.0 pcf; OMC = 10.0%.	t			10 10 11 11	LL=39, PL=12, F	I=27; P200=0
- - -				-wet layer at 10 feet.	- - - -			10 10 10 20		
2079.5	15.0	SP		POORLY GRADED SAND, fine-grained, trace G	- - ravel			12 11 12 11 3		
- - -		51		brown, damp.	- - -			3 2 2 2		
2074.5	20.0			END OF BORING.		4		2		
_				Bag sample collected from 3 to 15 feet.	-	-				
_				Water not observed with 20 feet of power auger in ground.	the	-				
_				Driller noted standing water in ditch.	-	-				
_				Boring then backfilled.		-				
_					-	-				
						-				
FA-12-00359				Braun Intertec Corporation						S-022 pag



		ect FA-1 Evaluation	2-00359 n	BORING		oti ==		S-023	71 250
Willist Severa	on NW I Routes		Williston	E117958	31.631 S	See S	9/80 ketch	+00; N4393	/1.338,
DRILLE	ER: J. E	Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1" =
Elev. feet 2101.6	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Note
2101.1	0.5		LEAN CLAY with SAND, roots and Organics, Gravel, brown, wet.  SANDY LEAN CLAY, trace Gravel, brown, m damp.  A-7-6 (19)  MDD = 127.0 pcf; OMC = 11.0%.  END OF BORING.  Bag sample collected from 0.5 to 15 feet.  Water not observed with 15 feet of power auger ground.  Boring then backfilled.	oist to			19 16 9 10 9 10 10 11 11 11 11	LL=44, PL=13, 1	Pl=31; P200
A-12-0035	9		Braun Intertec Corporation			1		LS	SS-023 p



Geotechnical Evaluation Williston NW Bypass Several Routes, NW of Williston Williams County, North Dakota  DRILLER: J. Brooks METHOD: Power Auger DATE:  Elev. feet feet ASTM 2105.4 0.0 Symbol (ASTM D2488 or D2487)  FILL Sandy Lean Clay, trace Gravel, brown, wet.  2103.4 2.0 SC CLAYEY SAND, a little Gravel, brown, moist.  A-6 (3) MDD = 137.0 pcf; OMC = 8.0%.	576.81 S	See Sk	9785 tetch.	SCALE: Tests  LL=26, PL=13, P	1" = 4' or Notes
DRILLER: J. Brooks  METHOD: Power Auger  DATE:  Elev. Depth feet feet ASTM 2105.4 0.0 Symbol  FILL FILL: Sandy Lean Clay, trace Gravel, brown, wet.  SC CLAYEY SAND, a little Gravel, brown, moist.  A-6 (3) MDD = 137.0 pcf; OMC = 8.0%.			MC % 12 9 12 9 10 9	Tests	or Notes
feet feet ASTM Description of Materials (ASTM D2488 or D2487)  FILL FILL: Sandy Lean Clay, trace Gravel, brown, wet.  2103.4 2.0  SC CLAYEY SAND, a little Gravel, brown, moist. A-6 (3) MDD = 137.0 pcf; OMC = 8.0%.	BPF	WL	%   12   9   12   9   10   9	LL=26, PL=13, P	
FILL FILL: Sandy Lean Clay, trace Gravel, brown, wet.  2103.4  2.0  SC  CLAYEY SAND, a little Gravel, brown, moist. A-6 (3) MDD = 137.0 pcf; OMC = 8.0%.			12 9 12 9 10 9	LL=26, PL=13, P	rI=13; P200=489
A-6 (3) MDD = 137.0 pcf; OMC = 8.0%.			9 10 9		PI=13; P200=489
2098.4 7.0 CL SANDY LEAN CLAY, trace Gravel, brown, moist.					
CL SANDY LEAN CLAY, trace Gravel, brown, moist.			"		
			14 14		
2095.4 10.0 END OF BORING.			13		
Bag sample collected from 2 to 7 feet.  Water not observed with 10 feet of power auger in the ground.	-				
Boring then backfilled.					
_					



	Costochmical Evaluation				BORING: LSS-025						
Willisto Several	n NW Routes	Bypass s, NW o	of Williston of Dakota	LOCATION: Station 9790+00; N440370.162, E1179547.453 See Sketch.							
	DRILLER: J. Brooks METHOD: Power Auger				3/1	2/13		SCALE:	1"=		
Elev. feet 2100.9	Depth feet 0.0	ASTM Symbo			BPF	WL	MC %	Tests	s or Note		
2100.9  2100.4  2090.9	10.0	CL CL	SANDY LEAN CLAY, trace Gravel, roots an brown, wet.  SANDY LEAN CLAY, trace Gravel, brown, 1 A-6 (14) MDD = 129.0 pcf; OMC = 10.0%.  END OF BORING.  Bag sample collected from 0.5 to 10 feet.  Water not observed with 10 feet of power aug ground.  Boring then backfilled.	moist			13 13 12 11 11 11 10 11 12 11	LL=40, PL=13,	PI=27; P200		
-				- - -							
_			Braun Intertec Corporation	_					SS-025		



	hnical l		1-12-00359	BORING						
Willist Severa	on NW I Route	Bypas s, NW		LOCATION: Station 9795+00; N440869.915, E1179539.35 See Sketch.						
DRILLE		Brooks	METHOD: Power Auger	DATE:	3/1	2/13		SCALE:	1"=	
Elev. feet 2092.1	Depth feet 0.0	ASTI Symb		•	BPF	WL	MC %	Tests	or Notes	
2091.6	0.5	CL	SANDY LEAN CLAY, trace Gravel, roots a brown, wet.	and Organics,			16	LL=30, PL=11, F	I=19; P200=	
_		CL	SANDY LEAN CLAY, trace Gravel, brown	, moist to			9			
			damp. A-6 (8)	_			6			
_			MDD = 132.0  pcf; OMC = 9.0%.	_			7			
							9			
_				_			9			
_				_			10			
_				_			11			
_				_			12			
2082.1	10.0		END OF DODAYS		1		12			
_			END OF BORING.	_	-					
-			Bag sample collected from 0.5 to 10 feet.		-					
_			Water not observed with 10 feet of power au ground.	iger in the	_					
-			Boring then backfilled.	_	1					
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	)		Braun Intertee Corpora						S-026 pa	



Geote	chnical 1	Evaluat		BORING: <b>LSS-027</b> LOCATION: Station 9800+00; N441369.552,					
Severa		s, NW	of Williston rth Dakota	E1179555.546 See Sketch.					
DRILLI		Elliot	METHOD: Power Auger	DATE:	3/2	3/13		SCALE:	1'' = 4
Elev. feet 2087.7	Depth feet 0.0	ASTM Symbo			BPF	WL	MC %	Tests	or Notes
2087.4			SANDY LEAN CLAY, with roots and organi moist.	cs, black,			11	LL=37, PL=16, F	PI=21; P200=6
_		CL	SANDY LEAN CLAY, trace Gravel, dark broad- A-6 (10) MDD = 123.0 pcf; OMC = 11.0%.	own, damp.			10 9 8		
2082.7	5.0	CL	SANDY LEAN CLAY, trace Gravel, brown,				5		
_		CL	SANDI LEAN CLAI, trace Graver, brown,				13		
_				-			15 15		
<del>-</del> 							16 18		
_				-			18		
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							17 18		
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2067.7	20.0		END OF BORING.		1		17		
_			Bag sample collected from 0.3 to 5 feet.	_	-				
_			Water not observed with 20 feet of power aug ground.	er in the	-				
_			Boring then backfilled.	-					
_				-					
_				-					
_					-				
- FA-12-0035			Braun Intertec Corporati	-					S-027 pag



		ect FA-1		BORING:			LS	S-028	
Willist Severa	chnical I ton NW al Routes ms Cour	LOCATION: Station 9805+00; N441869.078, E1179576.648 See Sketch.							
DRILLE	ER: C. I	Elliot	METHOD: Power Auger	DATE:	3/2	3/13		SCALE:	1" = 4'
Elev. feet 2106.2	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes
2105.9/ - - - - -		CL	SANDY LEAN CLAY, with roots and organics, moist.  SANDY LEAN CLAY, trace Gravel, brown, mo A-7-6 (17)  MDD = 122.0 pcf; OMC = 12.0%.				19 19 19 19 18 18 18 18	LL=44, PL=14, P	I=30; P200=675
2096.2	10.0		END OF BORING.  Bag sample collected from 0.3 to 10 feet.  Water not observed with 10 feet of power auger ground.  Boring then backfilled.	in the					
-				- - - -					
-				- - - -					
- 									



	Braun Project FA-12-00359 Geotechnical Evaluation BORING:											
Willist Severa	on NW al Route		Williston	LOCATION: Station 9810+00; N442368.53, E1179597.711 See Sketch.								
DRILLE		Elliot	METHOD: Power Auger	DATE:	3/23/13 SCAI				1"=			
Elev. feet 2112.2	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)	•	BPF	WL	MC %	Tests	or Note			
_2111.9 <i>/</i> _	0.3	CL	SANDY LEAN CLAY, with roots and organic moist. SANDY LEAN CLAY, trace Gravel, brown, n				17 17	LL=41, PL=14, I	PI=27; P200=			
_			A-7-6 (16) MDD = 123.0 pcf; OMC = 11.0%.				17 17					
_				_			17					
_				_			17 17					
_				-			18 18					
_							17					
_				_			16 16					
<u> </u>				_			17 16					
	15.0			_			16					
_			END OF BORING.  Bag sample collected from 0.3 to 15 feet.	_								
_			Water not observed with 15 feet of power augustion.	er in the								
_			Boring then backfilled.									
_				_								
_				_								
_												
_				_								
_				_								
_												
- FA-12-0035	0		Braun Intertee Corporatio	_					S-029 pa			



		ect FA-1		BORING:			LS	S-030	
Willist Severa	chnical I ton NW al Routes ms Cour	LOCATION: Station 9815+00; N442867.989, E1179618.927 See Sketch.							
DRILLE		Elliot	METHOD: Power Auger	DATE:	3/23	3/13		SCALE:	1'' = 4'
Elev. feet 2117.9	Depth feet 0.0	ASTM Symbol	Description of Materials (ASTM D2488 or D2487)		BPF	WL	MC %	Tests	or Notes
2117.6	5.0	CL CL CL	SANDY LEAN CLAY, with roots and organics, moist.  SANDY LEAN CLAY, trace Gravel, brown, dar A-6 (9)  MDD = 126.0 pcf; OMC = 9.0%.  SANDY LEAN CLAY, trace Gravel, brown, mo	np			7 6 6 6 9 14	LL=34, PL=13, P	I=21; P200=60%
2107.9	10.0		END OF BORING.	-			15 15 15 14		
· · · · · · · · · · · · · · · · · · ·			Bag sample collected from 0.3 to 5 feet.  Water not observed with 10 feet of power auger ground.  Boring then backfilled.	in the					
				- - - -					
				- - - -					
- 									

# BRAUN INTERTEC

## Descriptive Terminology of Soil



Standard D 2487 - 00 Classification of Soils for Engineering Purposes (Unified Soil Classification System)

	Criter	ia for Assigni	na Groun	Symbols and	Soi	ls Classification
	Gro	Group Symbol	Group Name <sup>b</sup>			
ر ق	Gravels	Clean G		$C_u \ge 4$ and $1 \le C_c \le 3^c$	GW	Well-graded graveld
grained Soils 50% retained o 200 sieve	More than 50% of coarse fraction	5% or less	fines °	C <sub>u</sub> < 4 and/or 1 > C <sub>c</sub> > 3 <sup>c</sup>	GP	Poorly graded gravel <sup>d</sup>
etaii eve	retained on	Gravels wit	th Fines	Fines classify as ML or MH	GM	Silty gravel of g
grained 50% reta 200 siev	No. 4 sieve	More than 12	2% fines *	Fines classify as CL or CH	GC	Clayey gravel dfg
20 50	Sands	% or more of 5% or less f		$C_u \ge 6$ and $1 \le C_o \le 3^c$	sw	Well-graded sand <sup>h</sup>
Coarse- nore than No.	50% or more of coarse fraction passes No. 4 sieve			C <sub>u</sub> < 6 and/or 1 > C <sub>c</sub> > 3 <sup>c</sup>	SP	Poorly graded sand h
Coa ore		Sands with	h Fines	Fines classify as ML or MH	SM	Silty sand <sup>fg h</sup>
Ĕ		More than	12% <sup>i</sup>	Fines classify as CL or CH	sc	Clayey sand fgh
ž	Silts and Clays	Inorganic	PI>7ar	nd plots on or above "A" line !	CL	Lean clay k l m
ed Soils passed the sieve		mo.ga.no	PI < 4 or plots below "A" line!		ML	Silt k i m
ad So passer sieve	less than 50	Organic	Liquid limit - oven dried < 0.75		OL	Organic clay k l m n
හි ල ස් වෙස			Liquid lin	nit - not dried	OL	Organic silt k 1 m o
graine more (	Silts and clays	Inorganic	Pi piots o	n or above "A" line	CH	Fat clay k i m
ne-g orn No.	Liquid limit	morganic	PI plots b	elow "A" line	MH	Elastic silt k I m
Fine-grained 50% or more pa No. 200 sie	50 or more	Organic	Liquid lim	nit - oven dried	ОН	Organic clay k i m p
50		Cryanic	Liquid lim	nit - not dried < 0.75	ОН	Organic sílt <sup>k t m q</sup>
Highly	Organic Soils	Primarily orga	anic matter	r, dark in color and organic odor	PT	Peat

- Based on the material passing the 3-in (75mm) sieve.
- If field sample contained cobbles or boulders, or both, add "with cobbles or boulders or both" to group name.
- $= D_{60}/D_{10} C_c = (D_{30})^2$

D<sub>10</sub> x D<sub>60</sub>

- If soil contains≥15% sand, add "with sand" to group name.
- Gravels with 5 to 12% fines require dual symbols:

GW-GM well-graded gravel with silt GW-GC well-graded gravel with clay

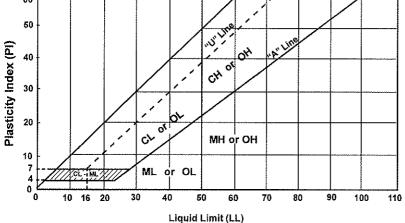
- GP-GM poorly graded gravel with silt GP-GC poorly graded gravel with clay If fines classify as CL-ML, use dual symbol GC-GM or SC-SM.
- If fines are organic, add "with organic fines" to group name.

  If soil contains ≥ 15% gravel, add "with gravel" to group name.
- Sands with 5 to 12% fines require dual symbols:

SW-SM well-graded sand with silt SW-SC well-graded sand with clay

- SP-SM poorly graded sand with silt
  SP-SC poorly graded sand with clay
  If Atterberg limits plot in hatched area, soil is a CL-ML, silty clay.
- If soil contains 10 to 29% plus No. 200, add "with sand" or "with gravel" whichever is predominant. If soil contains ≥ 30% plus No. 200, predominantly sand, add "sandy" to group name.
- If soil contains≥ 30% plus No. 200 predominantly gravel, add "gravelly" to group name
- n. Pl ≥ 4 and plots on or above "A" line.
- PI <4 or plots below "A" line.
- Pl plots on or above "A" line. q. Pi plots below "A" line.





#### Laboratory Tests

DD	Dry density, pcf	oc	Organic content, %
WD	Wet density, pcf	s	Percent of saturation, %
MC	Natural moisture content, %	SG	Specific gravity
LL	Liqiuid limit, %	C	Cohesion, psf
PL	Plastic limit, %	Ø	Angle of internal friction
Pl	Plasticity index, %	qu	Unconfined compressive strength, psf
P200	% passing 200 sieve	qp	Pocket penetrometer strength, tsf

#### Particle Size Identification

Boulders	over 12"
Cobbles	3" to 12"
Gravel	
Coarse	,
Fine	No. 4 to 3/4"
Sand	
Coarse	No. 4 to No. 10
Medium	No. 10 to No. 40
Fine	No. 40 to No. 200
Silt	< No. 200, PI < 4 or
	below "A" line
Clay	< No. 200, Pl≥ 4 and
	on or above "A" line

#### Relative Density of **Cohesionless Soils**

Very loose	0 to 4 BPF
Loose	5 to 10 BPF
Medium dense	11 to 30 BPF
Dense	31 to 50 BPF
Very dense	over 50 BPF

#### Consistency of Cohesive Soils

Very soft	0 to 1 BPF
Soft	2 to 3 BPF
Rather soft	4 to 5 BPF
Medium	6 to 8 BPF
Rather stiff	9 to 12 BPF
Stiff	13 to 16 BPF
Very stiff	17 to 30 BPF
Hard	over 30 BPF

#### **Drilling Notes**

Standard penetration test borings were advanced by 3 1/4" or 6 1/4" ID hollow-stem augers unless noted otherwise, Jetting water was used to clean out auger prior to sampling only where indicated on logs. Standard penetration test borings are designated by the prefix "ST" (Split Tube). All samples were taken with the standard 2" OD split-tube sampler, except where noted.

Power auger borings were advanced by 4" or 6" diameter continuousflight, solid-stem augers. Soil classifications and strata depths were inferred from disturbed samples augered to the surface and are, therefore, somewhat approximate. Power auger borings are designated by the

Hand auger borings were advanced manually with a 1 1/2" or 3 1/4" diameter auger and were limited to the depth from which the auger could be manually withdrawn. Hand auger borings are indicated by the prefix

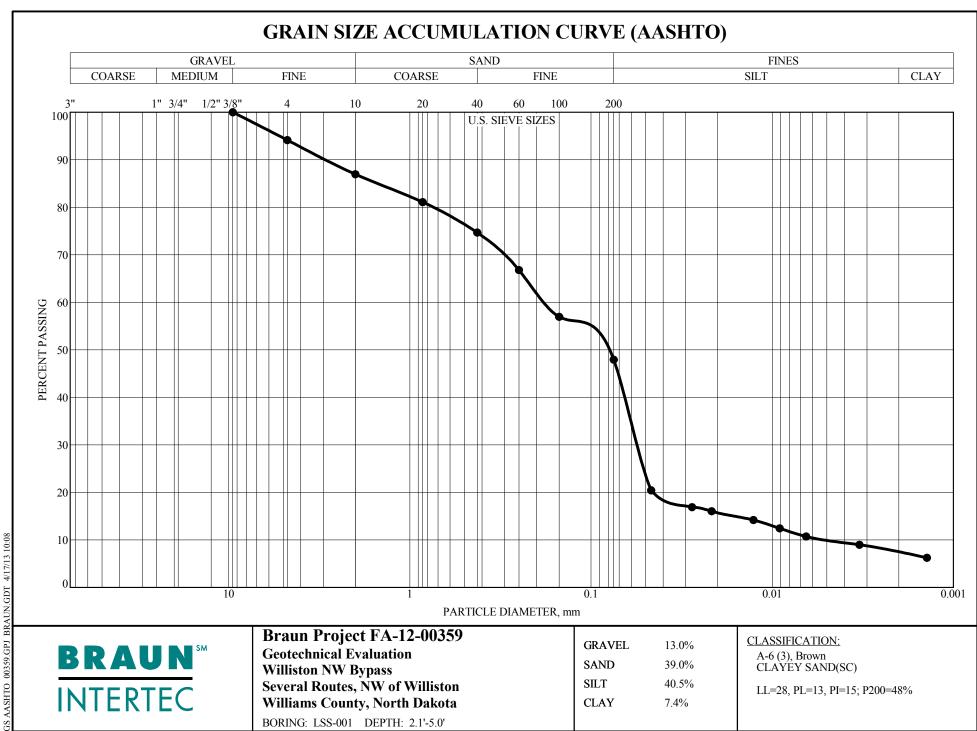
BPF: Numbers indicate blows per foot recorded in standard penetration test, also known as "N" value. The sampler was set 6" into undisturbed soil below the hollow-stem auger. Driving resistances were then counted for second and third 6" increments and added to get BPF. Where they differed significantly, they are reported in the following form: 2/12 for the second and third 6" increments, respectively,

WH: WH indicates the sampler penetrated soil under weight of hammer and rods alone; driving not required

WR: WR indicates the sampler penetrated soil under weight of rods alone; hammer weight and driving not required.

TW indicates thin-walled (undisturbed) tube sample.

Note: All tests were run in general accordance with applicable ASTM standards.



## GRAIN SIZE ACCUMULATION CURVE (AASHTO) GRAVEL SAND **FINES COARSE MEDIUM FINE COARSE** FINE SILT CLAY 3" 1" 3/4" 1/2" 3/8" 10 200 100 U.S. SIEVE SIZES 90 80 70 PERCENT PASSING 30 20 10 0.01 0.001 PARTICLE DIAMETER, mm **Braun Project FA-12-00359 CLASSIFICATION: GRAVEL** 46.8% **BRAUN**<sup>SM</sup> **Geotechnical Evaluation** A-2-6 (0), Brown CLAYEY SAND with GRAVEL(SC) **SAND** 31.8% Williston NW Bypass SILT 16.6% Several Routes, NW of Williston **INTERTEC** LL=24, PL=13, PI=11; P200=21% Williams County, North Dakota CLAY 4.8% BORING: LSS-002 DEPTH: 2.0'-5.0'

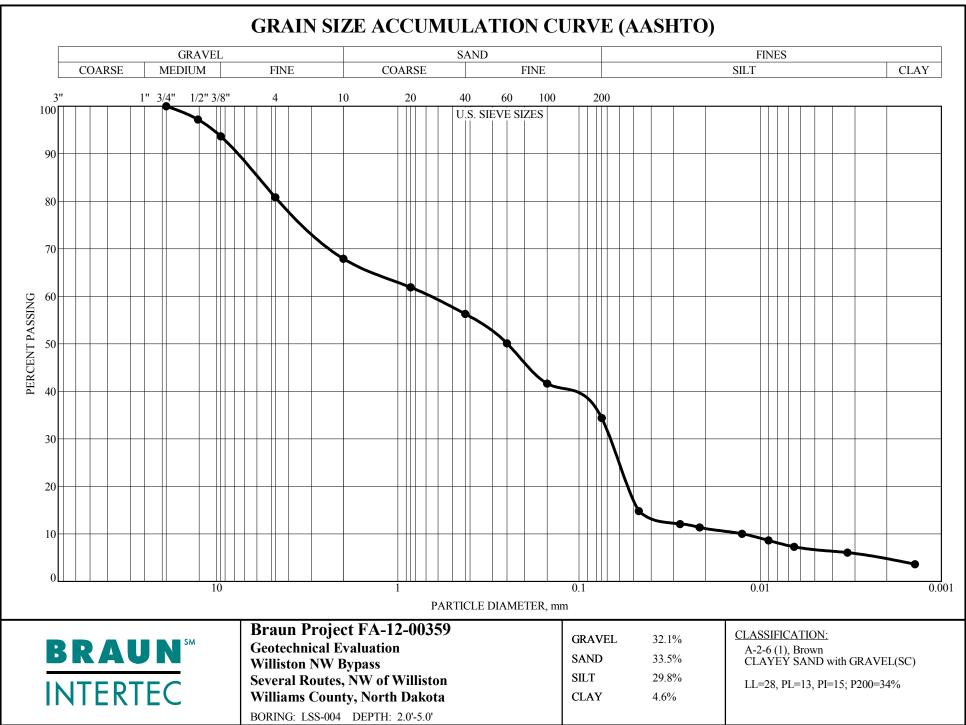
FA-12-00359

GS AASHTO 00359.GPJ BRAUN.GDT 4/17/13 10:08

## GRAIN SIZE ACCUMULATION CURVE (AASHTO) GRAVEL SAND **FINES COARSE MEDIUM FINE COARSE** FINE SILT CLAY 3" 1" 3/4" 1/2" 3/8" 10 200 100 U.S. SIEVE SIZES 90 80 70 PERCENT PASSING 30 20 10 0.01 0.001 PARTICLE DIAMETER, mm **Braun Project FA-12-00359 CLASSIFICATION: GRAVEL** 47.0% **BRAUN**<sup>SM</sup> **Geotechnical Evaluation** A-2-4 (0), Brown CLAYEY SAND with GRAVEL(SC) **SAND** 35.7% Williston NW Bypass SILT 12.6% Several Routes, NW of Williston **INTERTEC** LL=23, PL=14, PI=9; P200=17% Williams County, North Dakota CLAY 4.7% BORING: LSS-003 DEPTH: 2.0'-5.0'

FA-12-00359

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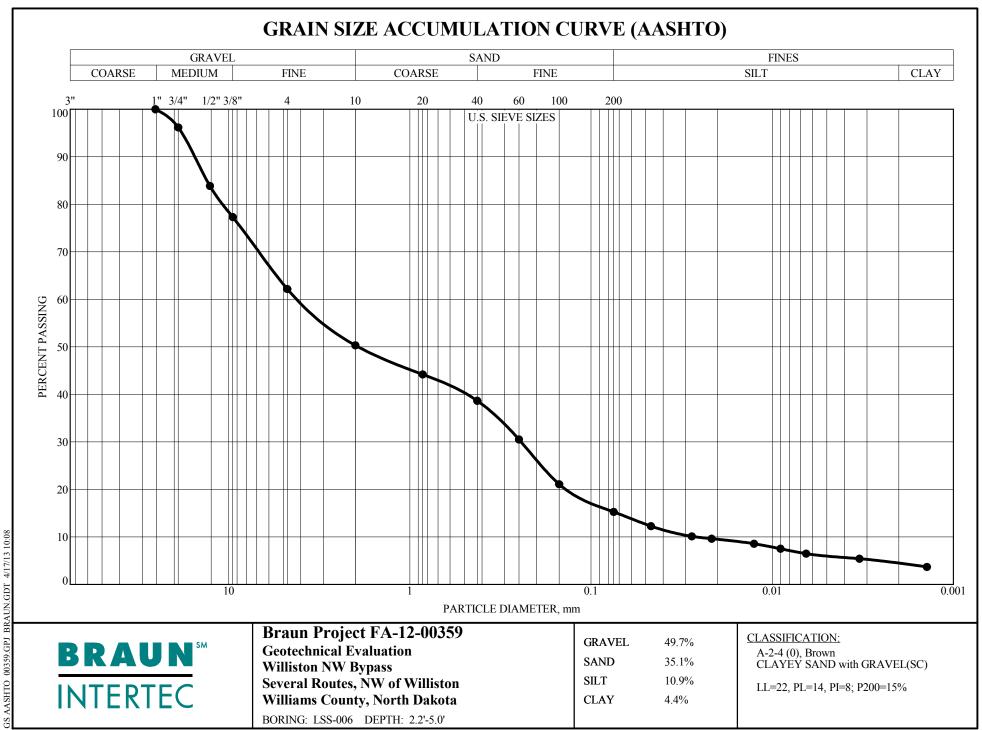


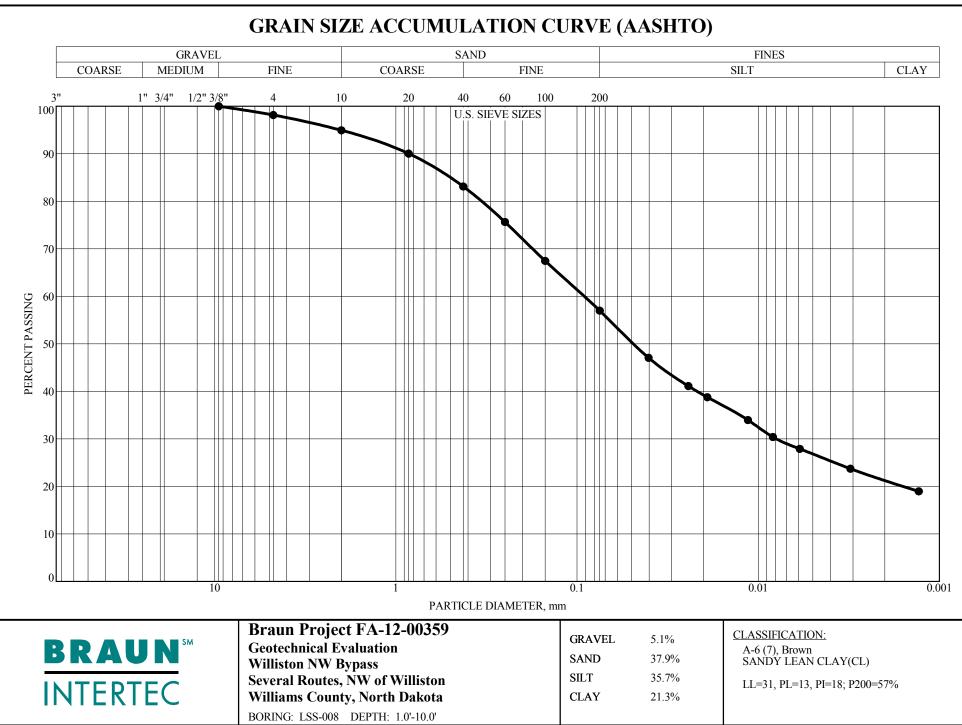
GS AASHTO 00359.GPJ BRAUN.GDT 4/17/13 10:08

## GRAIN SIZE ACCUMULATION CURVE (AASHTO) GRAVEL SAND **FINES COARSE MEDIUM FINE COARSE** FINE SILT CLAY 3" 1" 3/4" 1/2" 3/8" 10 200 100 U.S. SIEVE SIZES 90 80 70 PERCENT PASSING 30 20 10 0.01 0.001 PARTICLE DIAMETER, mm **Braun Project FA-12-00359 CLASSIFICATION: GRAVEL** 43.3% **BRAUN**<sup>SM</sup> **Geotechnical Evaluation** A-1-b (0), Brown SILTY SAND with GRAVEL(SM) **SAND** 39.0% Williston NW Bypass SILT 12.8% Several Routes, NW of Williston **INTERTEC** LL=NP, PL=NP, PI=NP; P200=18% Williams County, North Dakota CLAY 4.9% BORING: LSS-005 DEPTH: 2.5'-5.0'

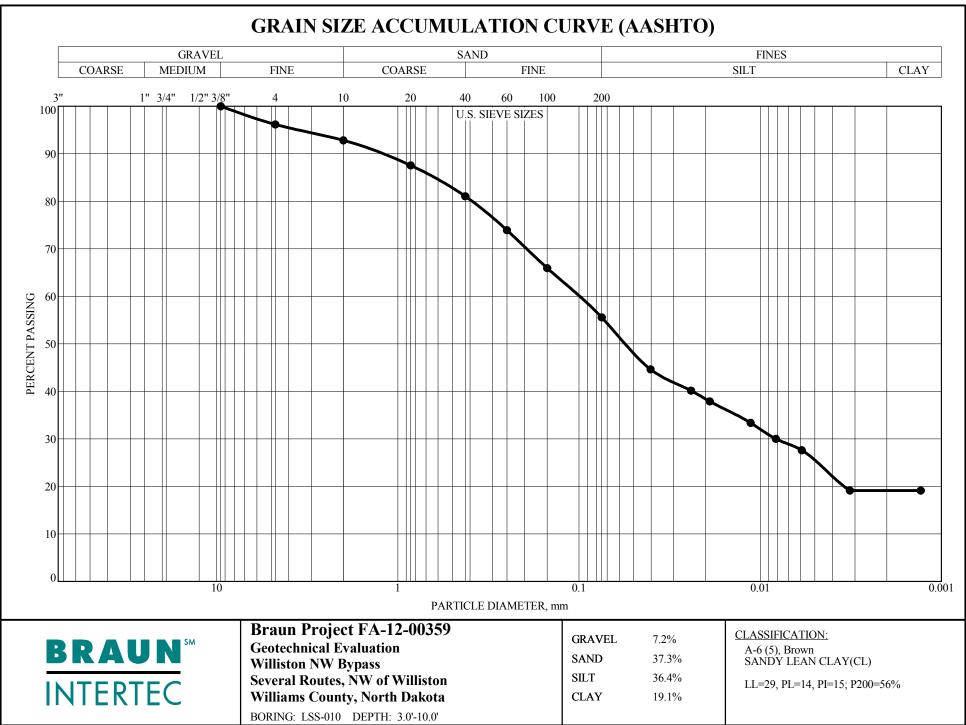
FA-12-00359

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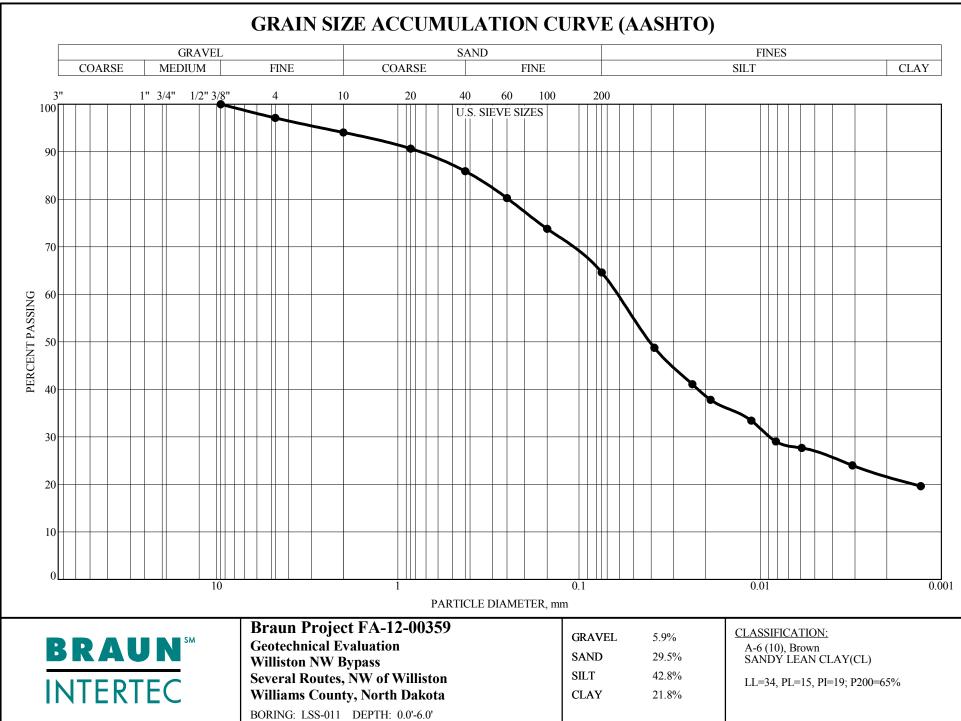




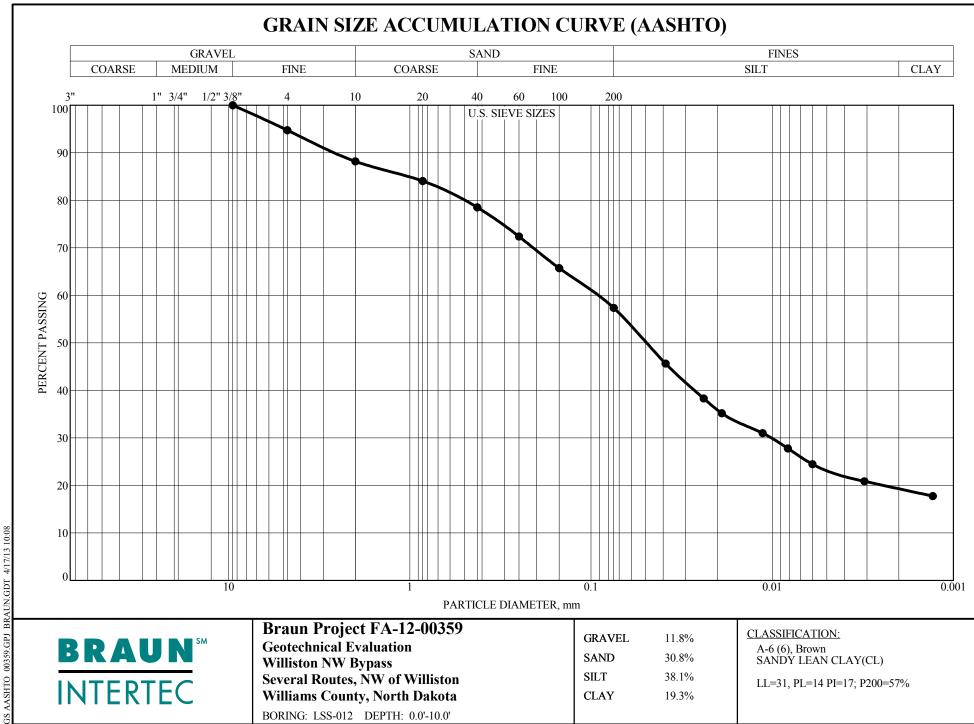
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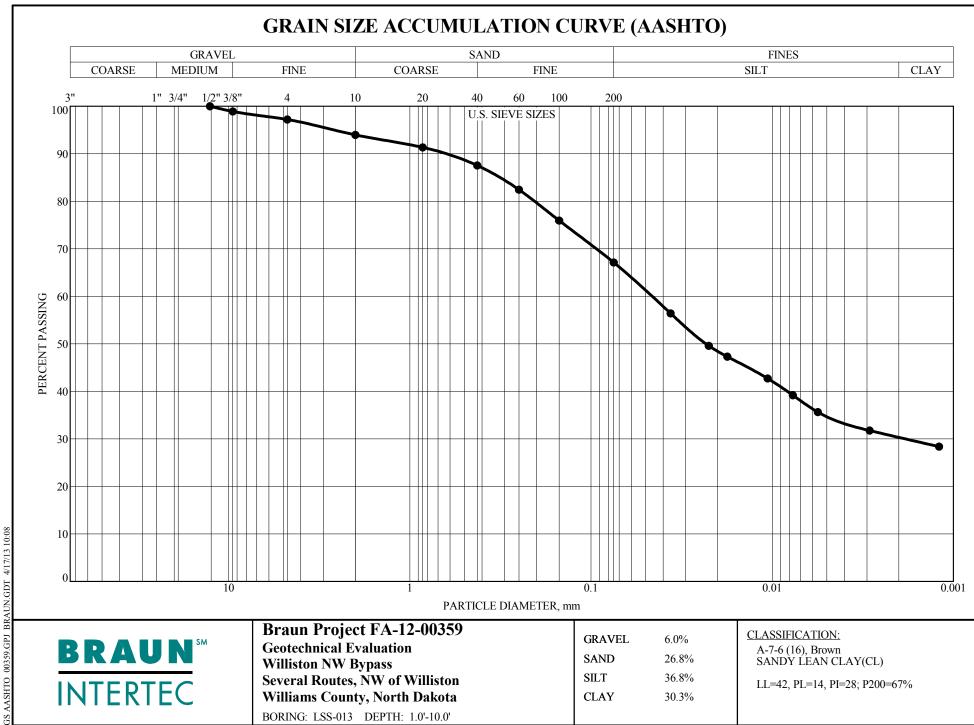


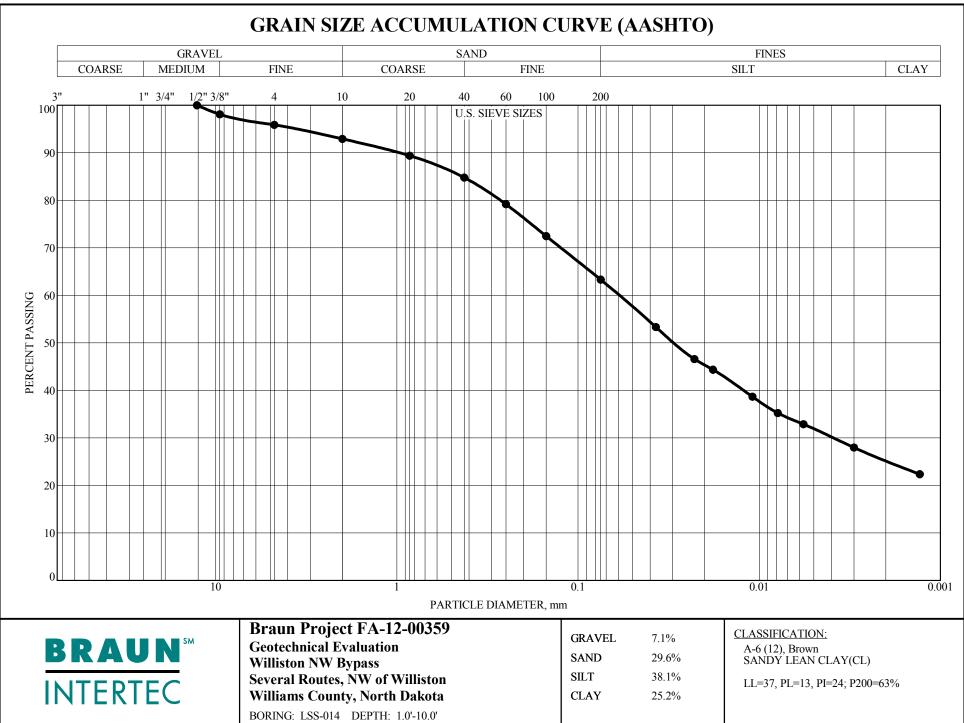
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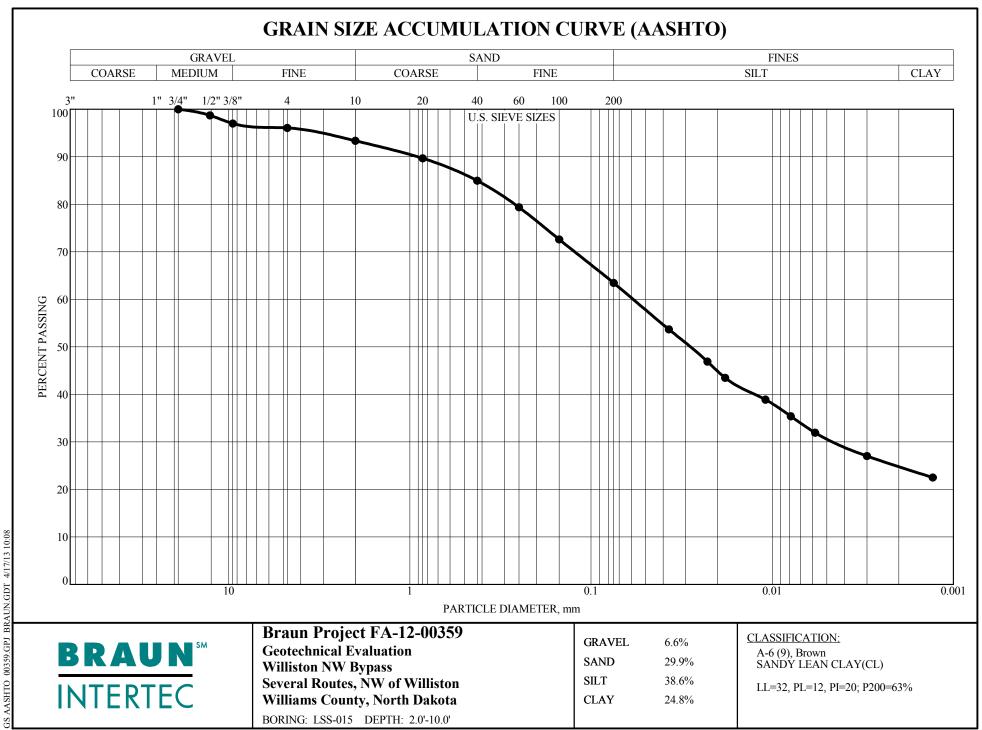
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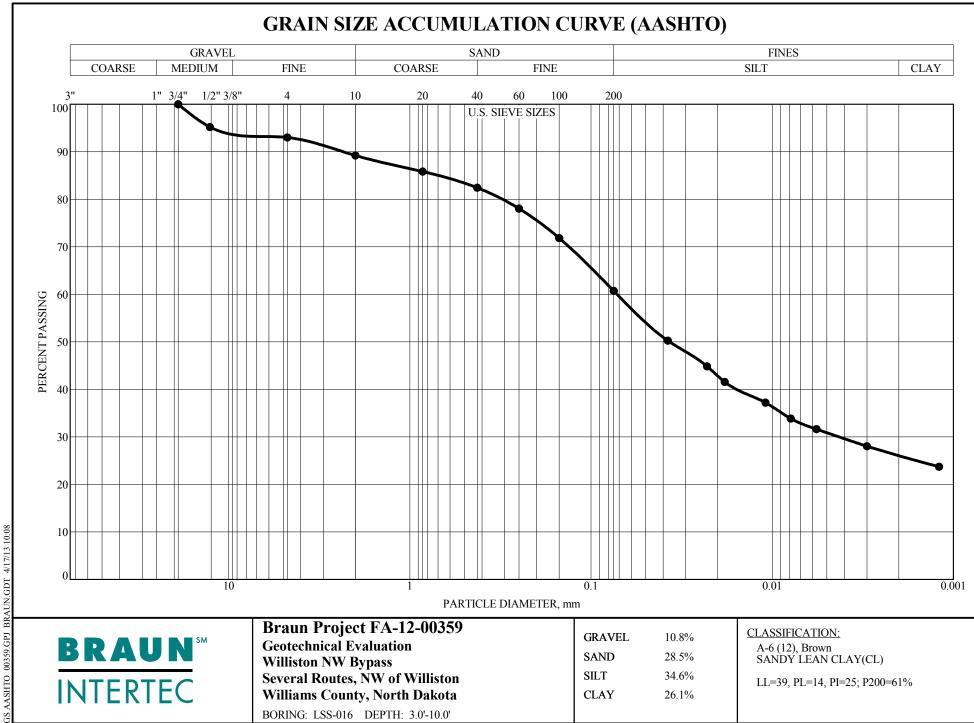


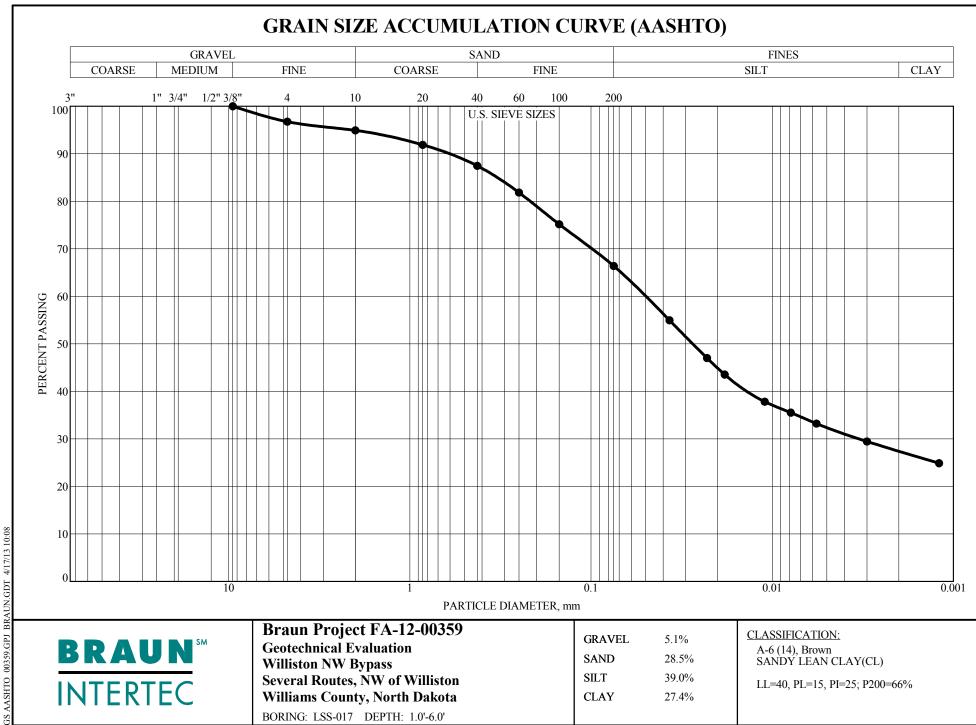


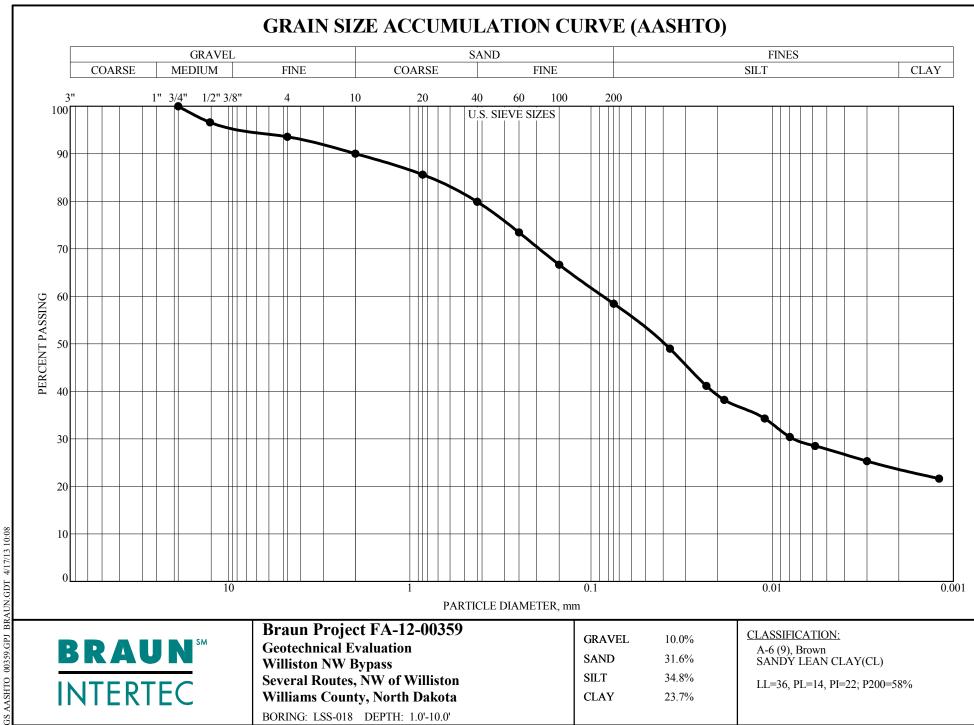


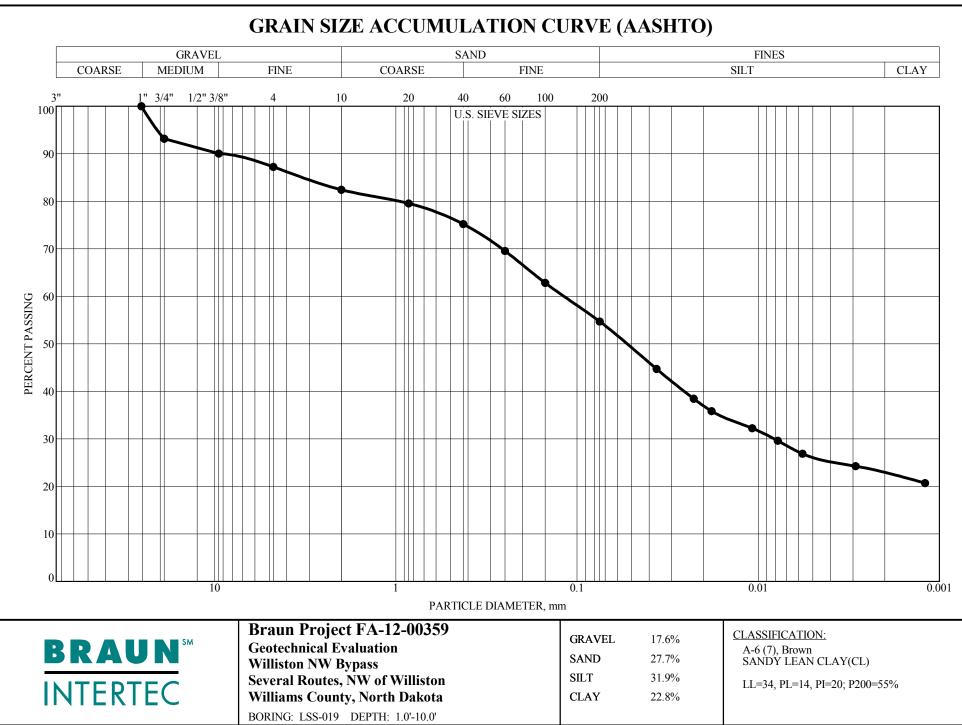
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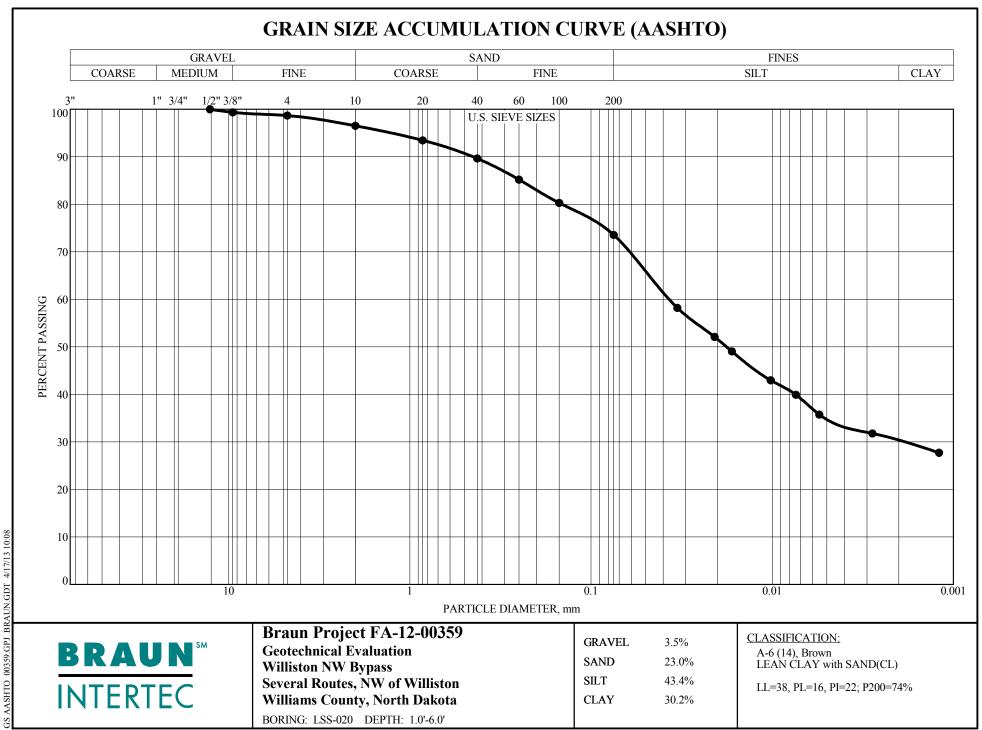


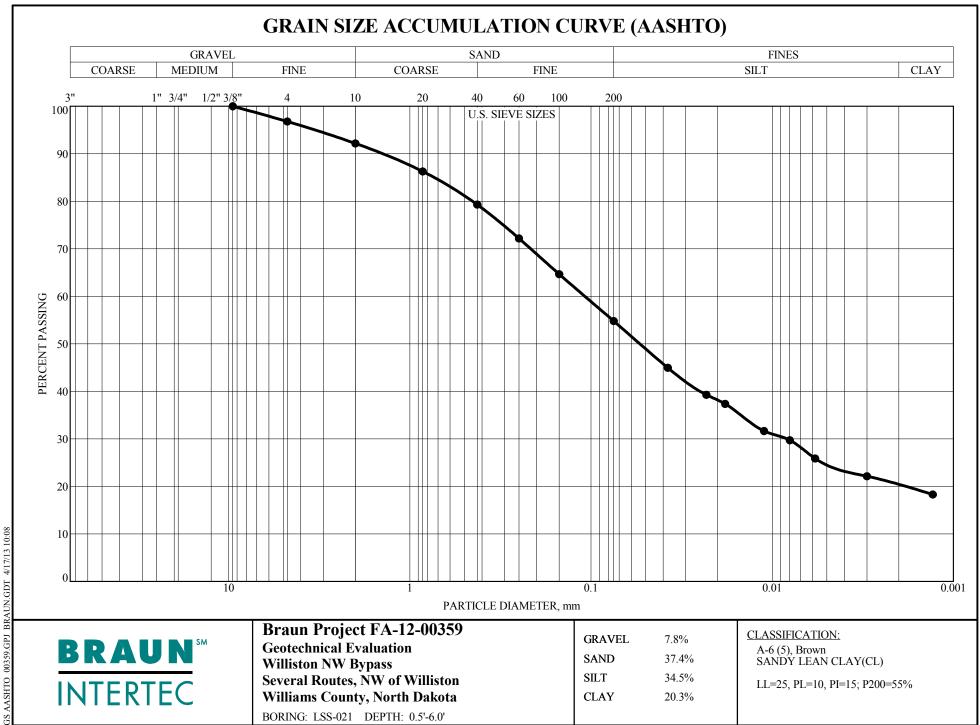


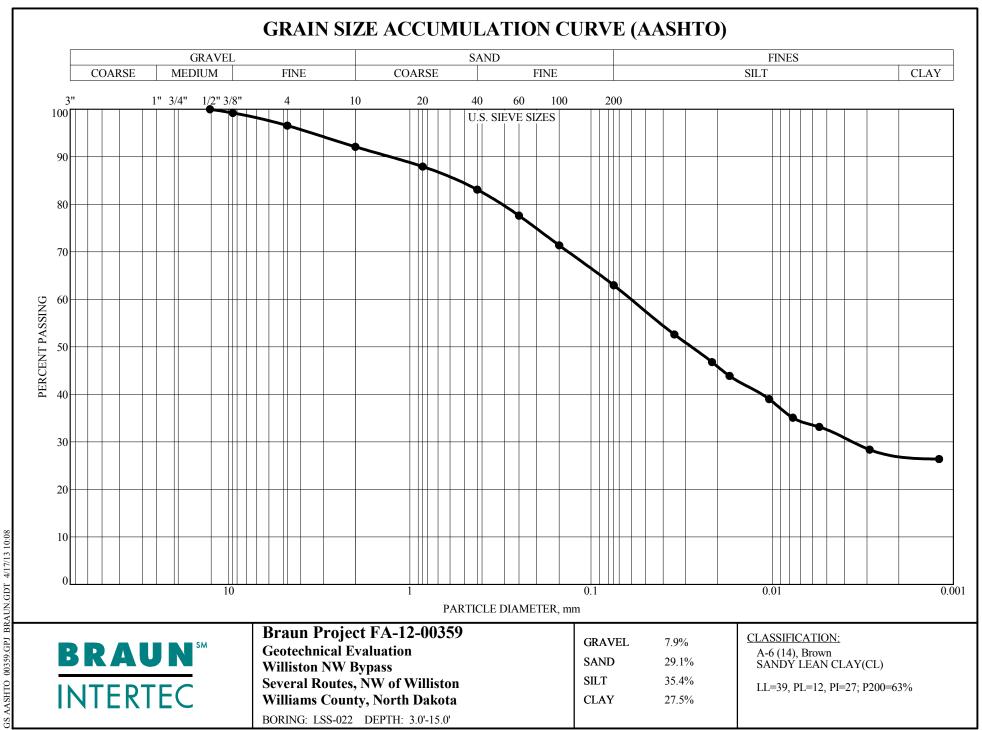


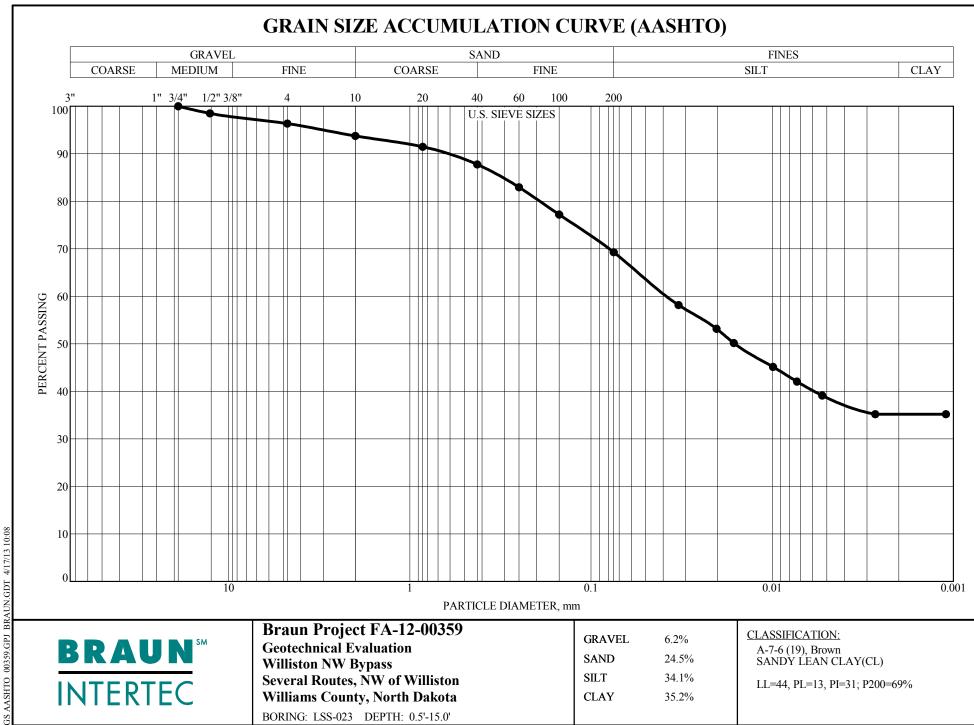


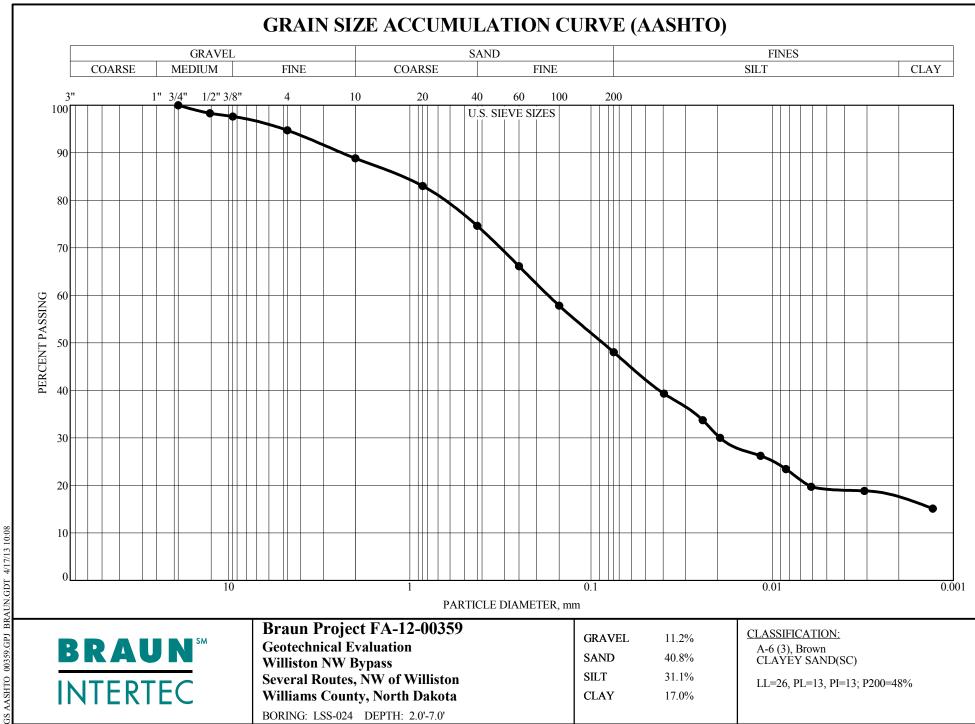
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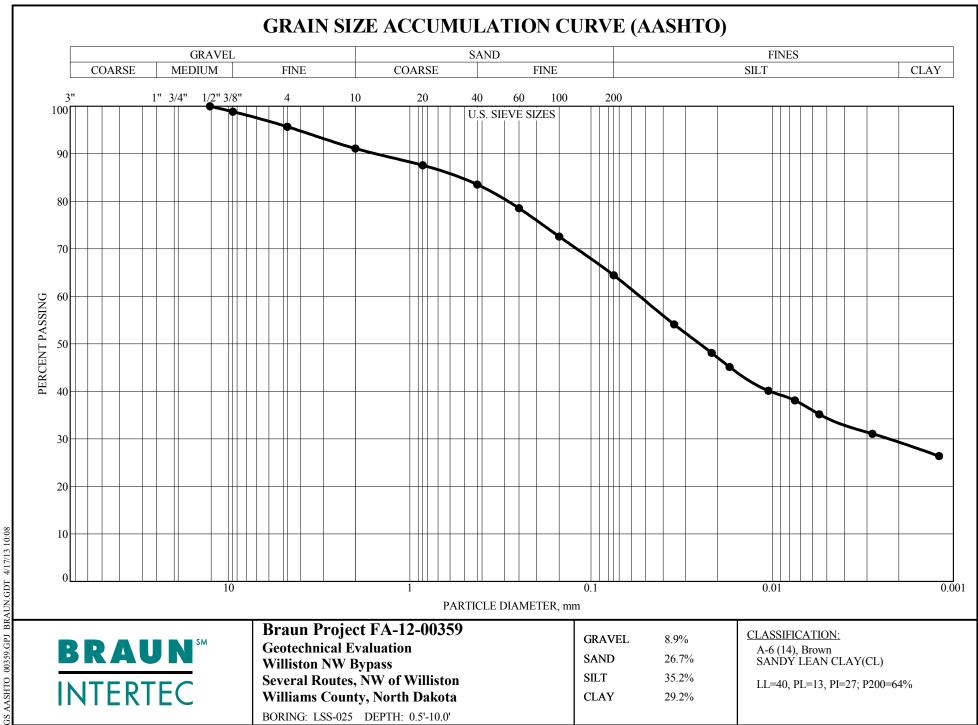


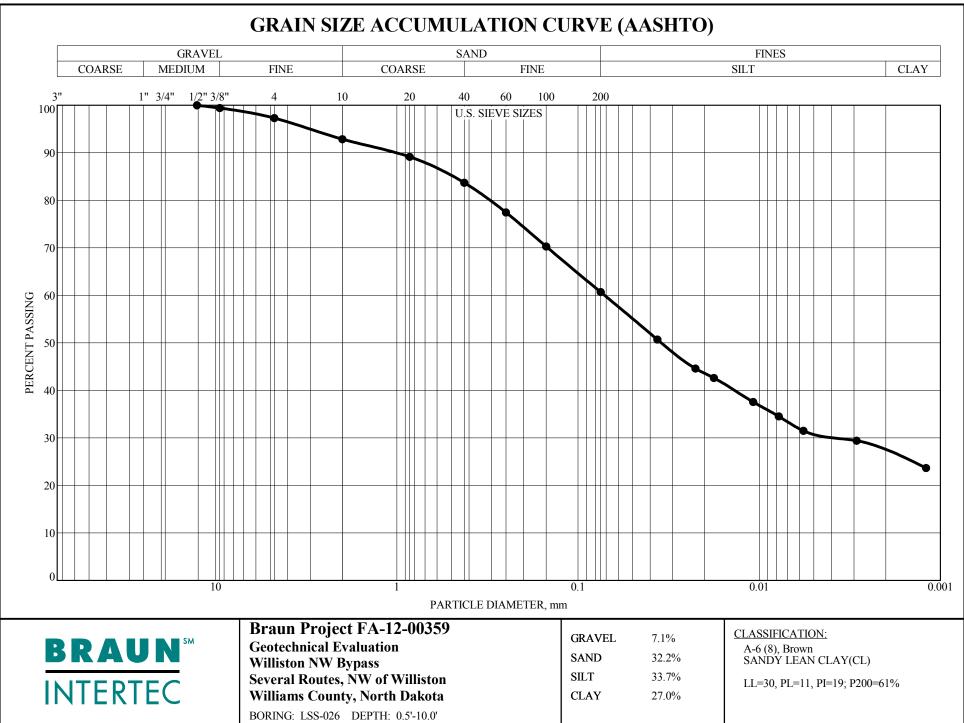




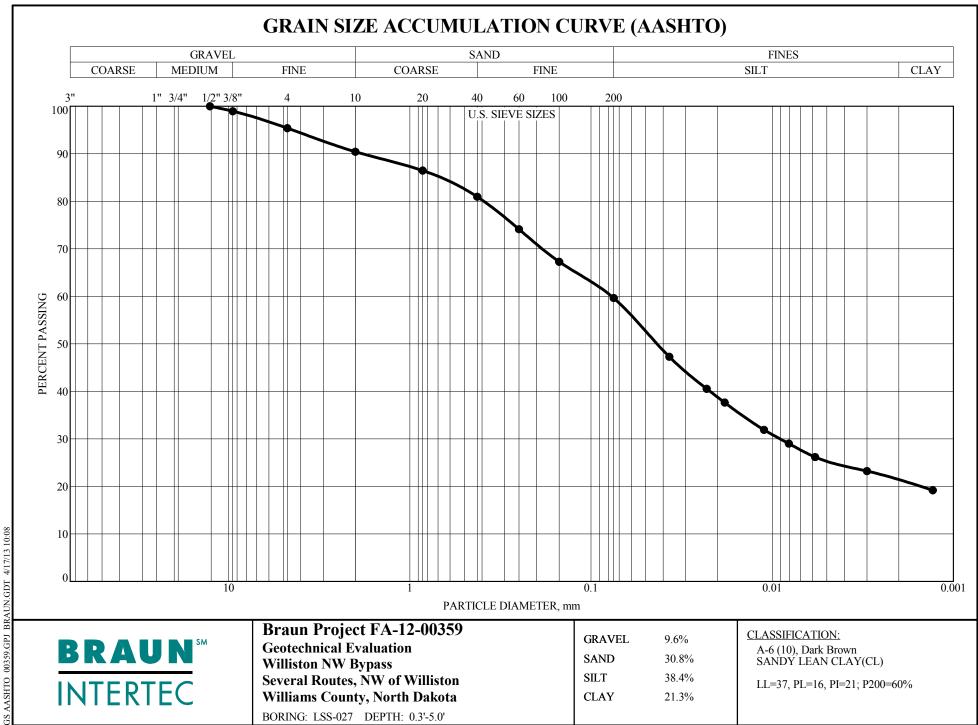


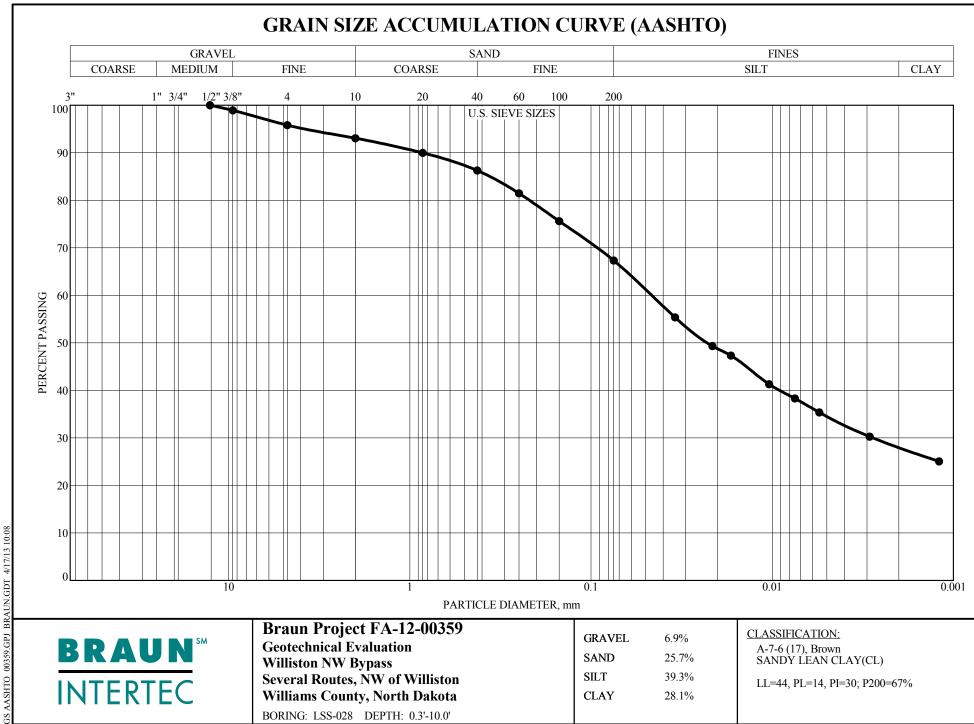


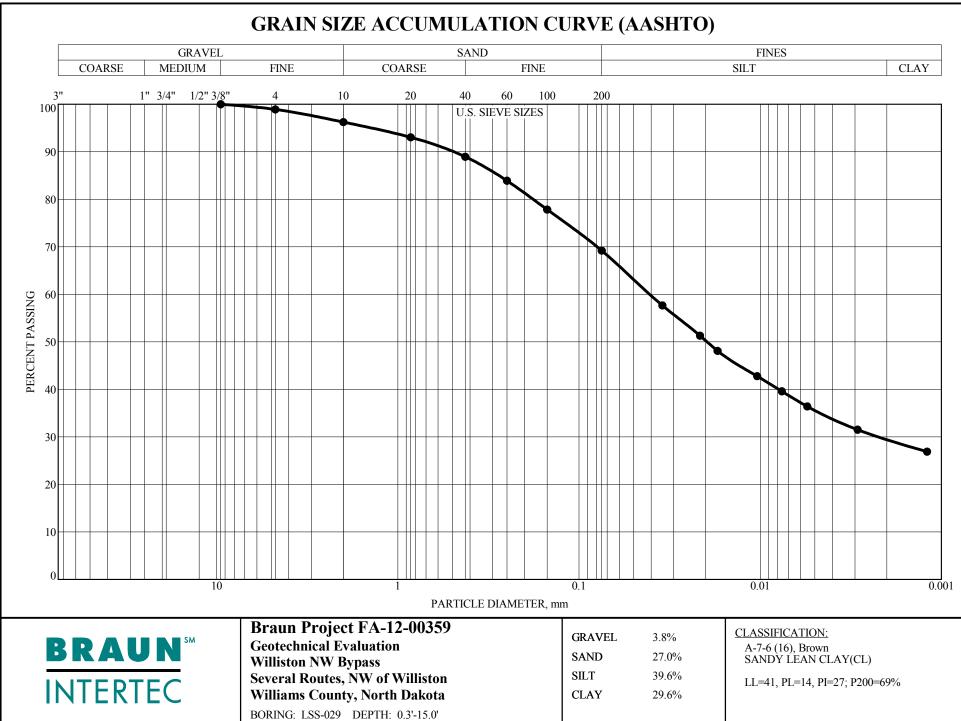




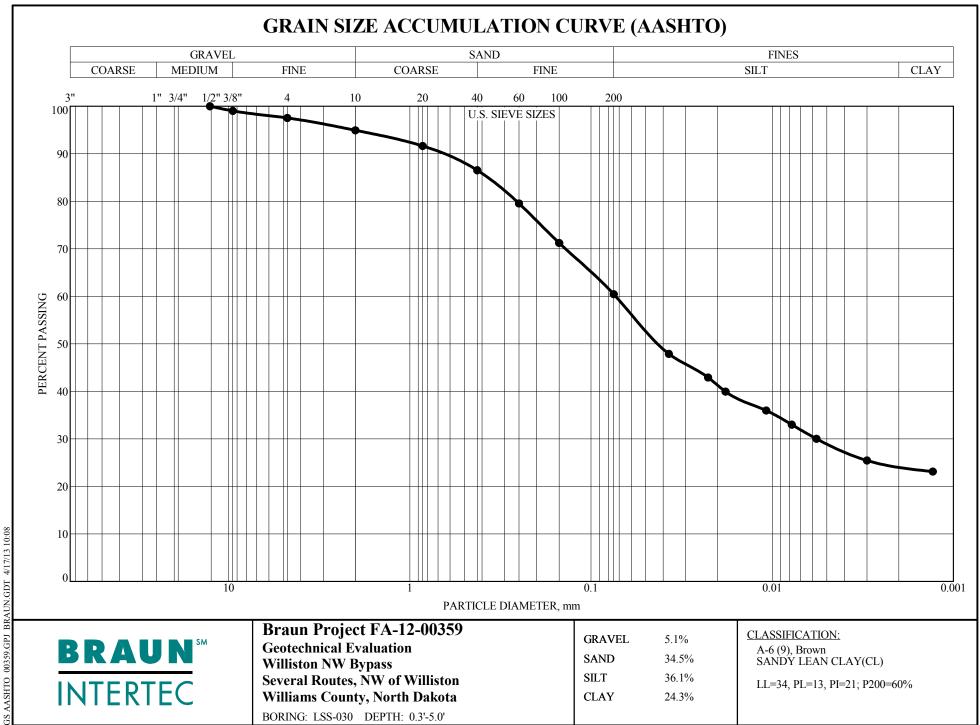
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1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S1

Brianne Nauman

EIT

Date of Issue: 4/9/2013

Sample Details

 Sample ID:
 W13-000494-S1

 Date Sampled:
 3/25/2013

 Sampled By:
 Steve Wenko

Source:Williston NW Bypass SubgradeMaterial:Clayey Sand (SC); A-6 (3)Specification:For Informational Purposes Only

Location: LSS-001; 2.1'-5' **Tested By:** Josh Beringer

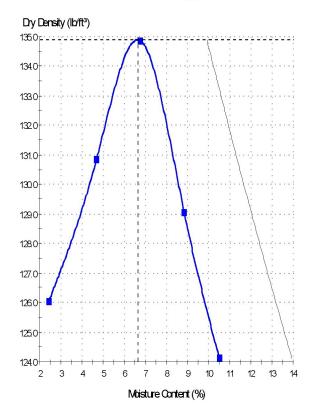
5/2013Date Submitted:3/25/2013ve WenkoSampling Method:Soil Boring Auger

**Date Tested:** 4/2/2013

Alternate Sample ID: LSS-001; 2.1'-5'



— 0% Air Voids



#### **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft³): 135 Corrected Maximum Dry Density 135

(lb/ft³):

Optimum Moisture Content (%): 7
Corrected Optimum Moisture 7

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 28; PI = 15

Percent Retained on #4 Sieve = 6.2%; Percent Passing #200 Sieve = 47.7%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza Fargo, ND, 58102

**Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S2 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/9/2013

Sample Details

Sample ID: W13-000494-S2 Date Sampled: 3/25/2013 Sampled By: Steve Wenko

Source: Williston NW Bypass Subgrade

Material: Clayey Sand with Gravel (SC); A-2-6 (0)

Specification: For Informational Purposes Only

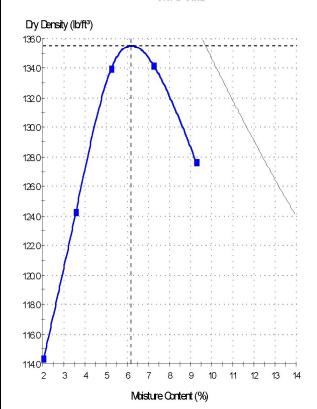
Location: LSS-002; 2'-5' Tested By: Josh Beringer

Alternate Sample ID: LSS-002; 2'-5' **Date Submitted:** 3/25/2013 Sampling Method: Soil Boring Auger

**Date Tested:** 4/2/2013



0% Air Voids



#### **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 136 **Corrected Maximum Dry Density** 136

(lb/ft³):

**Optimum Moisture Content (%):** 6 **Corrected Optimum Moisture** 6 Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 24; PI = 11

Percent Retained on #4 Sieve = 29.7%; Percent Passing #200 Sieve = 21.4%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston

Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S3
Issue No: 1

Brianne Nauman

EIT

Date of Issue: 4/9/2013

#### Sample Details

Sample ID:W13-000494-S3Alternate Sample ID: LSS-003; 2'-5'Date Sampled:3/25/2013Date Submitted: 3/25/2013Sampled By:Steve WenkoSampling Method: Soil Boring Auger

**Source:** Williston NW Bypass Subgrade

Material: Clayey Sand with Gravel (SC); A-2-4 (0)

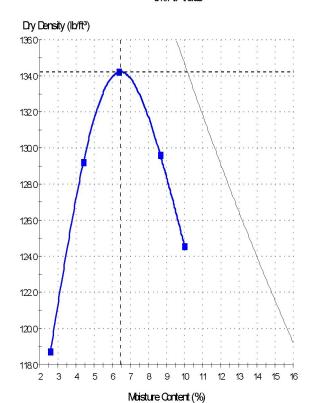
**Specification:** For Informational Purposes Only

Location: LSS-003; 2'-5'
Tested By: Josh Beringer

**Date Tested:** 

Dry Density - Moisture Content Relationship





#### Test Results

4/8/2013

Maximum Dry Density (lb/ft³): 134
Corrected Maximum Dry Density 134

(lb/ft³):

Optimum Moisture Content (%): 6
Corrected Optimum Moisture 6
Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 23; PI = 9

Percent Retained on #4 Sieve = 30.7%; Percent Passing #200 Sieve = 17.3%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

**PM:** Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S4 Issue No: 1

Brianne Nauman

133

EIT

Date of Issue: 4/9/2013

Sample Details

**Sample ID:** W13-000494-S4 **Date Sampled:** 3/25/2013

Sampled By: Steve Wenko

Source: Williston NW Bypass Subgrade

Material: Clayey Sand with Gravel (SC); A-2-6 (1)

**Specification:** For Informational Purposes Only

Location: LSS-004; 2'-5' **Tested By:** Josh Beringer

**Date Tested:** 4/5/2013

**Date Submitted:** 

Sampling Method:

Alternate Sample ID: LSS-004; 2'-5'

3/25/2013

**Test Results** 

Soil Boring Auger

Maximum Dry Density (lb/ft3):

# Dry Density - Moisture Content Relationship O%Air Voids Dry Density (lb/ff³) 1340 1320 1260 1260 1220

# Corrected Maximum Dry Density (lb/ft³): Optimum Moisture Content (%): Corrected Optimum Moisture Content (%): Method: D Removed

AASHTO T 180 - 01

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

120.0

118.0

Assumed Specific Gravity = 2.75

LL = 28; PI = 15

Percent Retained on #4 Sieve = 19.2%; Percent Pasing #200 Sieve = 34.4%

Moisture Content (%)



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S5 Issue No: 1

Brianne Nauman

D

**EIT** 

Date of Issue: 4/9/2013

#### Sample Details

Sample ID: W13-000494-S5 Date Sampled: 3/25/2013 Sampled By: Steve Wenko

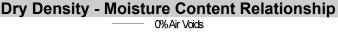
Source: Williston NW Bypass Subgrade Material: Silty Sand with Gravel (SM); A-1-b (0) Specification: For Informational Purposes Only

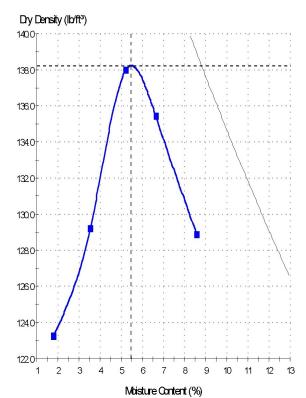
Location: LSS-005; 2.5'-5' Tested By: Josh Beringer

**Date Submitted:** 3/25/2013 Sampling Method: Soil Boring Auger

> **Date Tested:** 4/1/2013

Alternate Sample ID: LSS-005; 2.5'-5'





#### **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 138 **Corrected Maximum Dry Density** 138

(lb/ft³):

**Optimum Moisture Content (%):** 5 **Corrected Optimum Moisture** 5 Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

Non-Plastic

Percent Retained on #4 Sieve = 30.2%; Percent Passing #200 Sieve = 17.7%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S6 Issue No: 1

Brianne Nauman

137

**EIT** 

Date of Issue: 4/9/2013

#### Sample Details

Sample ID: W13-000494-S6 Alternate Sample ID: LSS-006; 2.2'-5' Date Sampled: 3/25/2013 **Date Submitted:** 3/25/2013 Sampled By: Steve Wenko Sampling Method: Soil Boring Auger

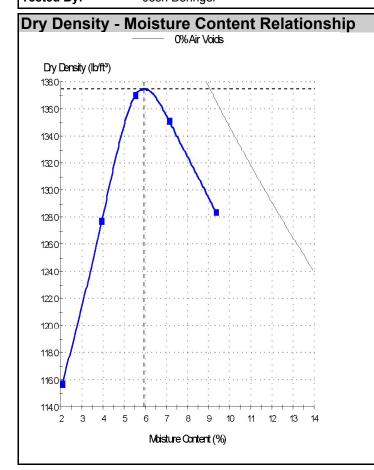
Source: Williston NW Bypass Subgrade

Material: Clayey Sand with Gravel (SC); A-2-4 (0)

Specification: For Informational Purposes Only

Location: LSS-006; 2.2'-5' Tested By: Josh Beringer

**Date Tested:** 4/2/2013



#### **Test Results**

AASHTO T 180 - 01 Maximum Dry Density (lb/ft3):

Corrected Maximum Dry Density 137

(lb/ft³):

**Optimum Moisture Content (%):** 6 **Corrected Optimum Moisture** 6 Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 22; PI = 8

Percent Retained on #4 Sieve = 37.8%; Percent Passing #200 Sieve = 15.3%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S8 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

#### Sample Details

Sample ID: W13-000494-S8 Date Sampled: 3/12/2013 Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (7) Specification: For Informational Purposes Only

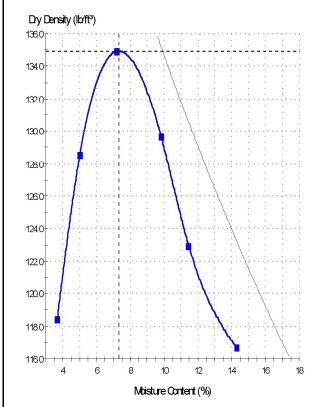
Location: LSS-008; 1'-10' Tested By: **Thomas Wagner** 

Alternate Sample ID: LSS-008; 1'-10' **Date Submitted:** 3/14/2013 Sampling Method: Soil Boring Auger

**Date Tested:** 3/19/2013

# **Dry Density - Moisture Content Relationship**





#### **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 135 **Corrected Maximum Dry Density** 135

(lb/ft³):

**Optimum Moisture Content (%):** 7 **Corrected Optimum Moisture** Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 31; PI = 18

Percent Retained on #4 Sieve = 7.2%; Percent Passing #200 Sieve = 57.0%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass Several Routes, NW of Williston

Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S10 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

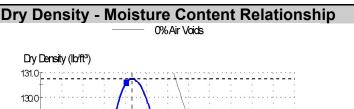
Sample ID: W13-000494-S10

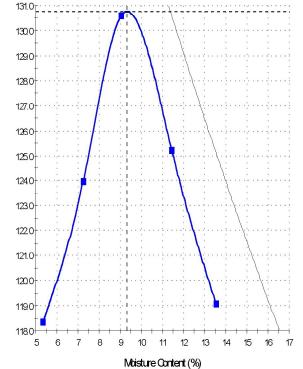
Date Sampled: 3/12/2013 Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (5) Specification: For Informational Purposes Only

Location: LSS-010; 3'-10'

Tested By: Kevin Ficek **Date Tested:** 





#### **Test Results**

3/14/2013

3/19/2013

Soil Boring Auger

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 131 Corrected Maximum Dry Density 131

(lb/ft³):

Alternate Sample ID: LSS-010; 3'-10'

**Date Submitted:** 

Sampling Method:

**Optimum Moisture Content (%):** 9 **Corrected Optimum Moisture** 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 29; PI = 15

Percent Retained on #4 Sieve = 5.4%; Percent Passing #200 Sieve = 55.6%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S11 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

#### Sample Details

Sample ID: W13-000494-S11 Date Sampled: 3/12/2013

Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (10) Specification: For Informational Purposes Only

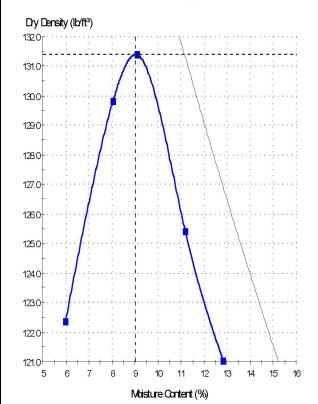
Location: LSS-011: 0'-6' Tested By: Kevin Ficek

Alternate Sample ID: LSS-011; 0'-6' **Date Submitted:** 3/14/2013 Sampling Method: Soil Boring Auger

**Date Tested:** 3/19/2013

# **Dry Density - Moisture Content Relationship**

0% Air Voids



#### **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 131 Corrected Maximum Dry Density 131

(lb/ft³):

**Optimum Moisture Content (%):** 9 **Corrected Optimum Moisture** 9 Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 34; PI = 19

Percent Retained on #4 Sieve = 5.7%; Percent Passing #200 Sieve = 64.6%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S12 Issue No: 1

Brianne Nauman

EIT

Date of Issue: 4/5/2013

Sample Details

**Sample ID:** W13-000494-S12

Date Sampled: 3/12/2013 Sampled By: John Brooks

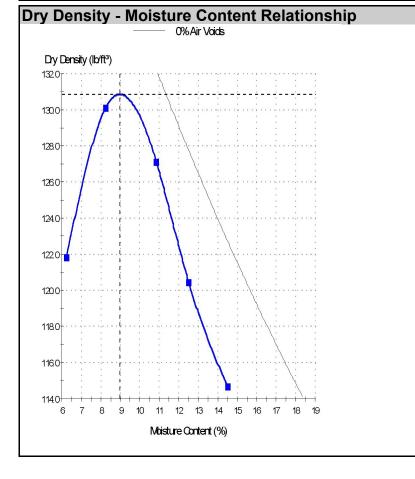
Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (6)

Specification: For Informational Purposes Only

Location: LSS-012; 0'-10'
Tested By: Thomas Wagne

Thomas Wagner Date Tested: 3/20/2013



#### Test Results

3/14/2013

Soil Boring Auger

AASHTO T 180 - 01 -

Maximum Dry Density (lb/ft³): 131 Corrected Maximum Dry Density 131

(lb/ft³):

Alternate Sample ID: LSS-012; 0'-10'

**Date Submitted:** 

Sampling Method:

Optimum Moisture Content (%): 9
Corrected Optimum Moisture 9
Content (%):

----- (70)

Method: D

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 31; PI = 17

Percent Retained on #4 Sieve = 5.3%; Percent Passing #200 Sieve = 57.4%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S13
Issue No: 1

Brianne Nauman

EIT

Date of Issue: 4/5/2013

Sample Details

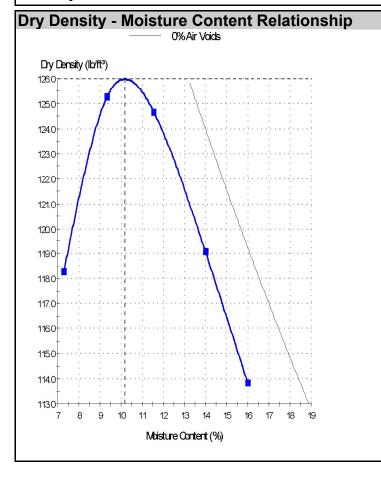
**Sample ID:** W13-000494-S13 **Date Sampled:** 3/12/2013

Sampled By: John Brooks

Source:Williston NW Bypass SubgradeMaterial:Sandy Lean Clay (CL); A-7-6 (16)Specification:For Informational Purposes Only

Location: LSS-013; 1'-10
Tested By: Matt Daavetilla

Daavetilla Date Tested: 3/20/2013



#### Test Results

3/14/2013

Soil Boring Auger

AASHTO T 180 - 01 -

Maximum Dry Density (lb/ft³): 126 Corrected Maximum Dry Density 126

(lb/ft³):

Alternate Sample ID: LSS-013; 1'-10

**Date Submitted:** 

Sampling Method:

Optimum Moisture Content (%): 10
Corrected Optimum Moisture 10

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 42; PI = 28

Percent Retained on #4 Sieve = 2.8%; Percent Passing #200 Sieve = 67.1%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S14

Brianne Nauman

EIT

Date of Issue: 4/5/2013

Sample Details

**Sample ID:** W13-000494-S14

Date Sampled: 3/12/2013 Sampled By: John Brooks

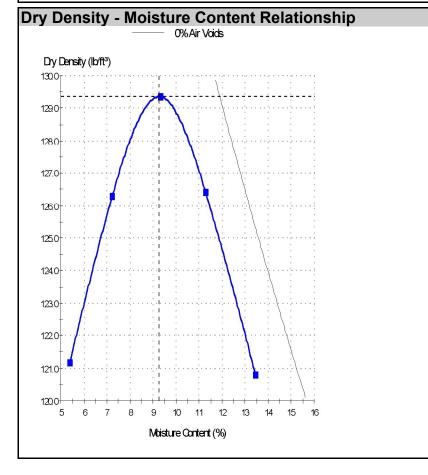
Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (12)

Specification: For Informational Purposes Only

Location: LSS-14; 1'-10

Tested By: Rodney Schnieder Date Tested: 3/20/2013



#### Test Results

3/14/2013

Soil Boring Auger

\_ AASHTO T 180 - 01 \_

Maximum Dry Density (lb/ft³): 129 Corrected Maximum Dry Density 129

(lb/ft³):

Alternate Sample ID: LSS-14; 1'-10

**Date Submitted:** 

Sampling Method:

Optimum Moisture Content (%): 9
Corrected Optimum Moisture 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round

Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 37; PI = 24

Percent Retained on #4 Sieve = 4.1%; Percent Passing #200 Sieve = 63.3%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S15 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

Sample ID: W13-000494-S15

Date Sampled: 3/12/2013

Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (9) Specification: For Informational Purposes Only

Location: LSS-15; 2'-10'

Tested By: Shane Falwey Alternate Sample ID: LSS-15; 2'-10'

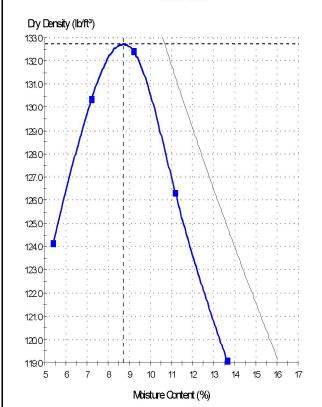
**Date Submitted:** 3/14/2013

**Date Tested:** 

Sampling Method: Soil Boring Auger



0% Air Voids



#### **Test Results**

3/20/2013

AASHTO T 180 - 01

Maximum Dry Density (lb/ft³): 133 **Corrected Maximum Dry Density** 133

(lb/ft³):

**Optimum Moisture Content (%):** 9 **Corrected Optimum Moisture** 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 32; PI = 20

Percent Retained on #4 Sieve = 3.9%; Percent Passing #200 Sieve = 63.4%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S16

Brianne Nauman

EIT

Date of Issue: 4/5/2013

Sample Details

**Sample ID:** W13-000494-S16

**Date Sampled:** 3/12/2013

Sampled By: John Brooks

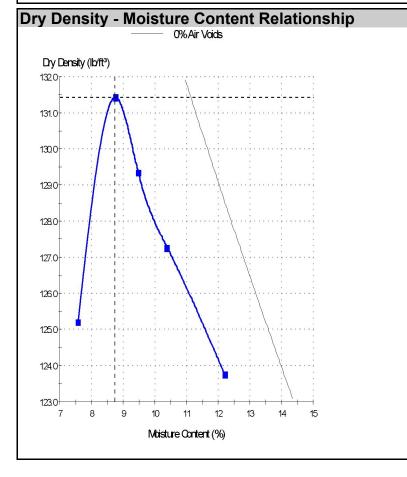
Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (12)

Specification: For Informational Purposes Only

Location: LSS-16; 3'-10'

Tested By: Kevin Ficek Date Tested: 3/19/2013



#### Test Results

3/15/2013

Soil Boring Auger

AASHTO T 180 - 01 -

Maximum Dry Density (lb/ft³): 131 Corrected Maximum Dry Density 131

(lb/ft³):

Alternate Sample ID: LSS-16; 3'-10'

**Date Submitted:** 

Sampling Method:

Optimum Moisture Content (%): 9
Corrected Optimum Moisture 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 39; PI = 25

Percent Retained on #4 Sieve = 7.0%; Percent Passing #200 Sieve = 60.7%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S17 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

Sample ID: W13-000494-S17 Date Sampled: 3/12/2013

Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (14) Specification: For Informational Purposes Only

Location: LSS-17; 1'-6' Tested By:

Kevin Ficek

Alternate Sample ID: LSS-17; 1'-6' **Date Submitted:** 3/14/2013 Sampling Method: Soil Boring Auger

**Date Tested:** 3/20/2013

# **Dry Density - Moisture Content Relationship** 0% Air Voids Dry Density (lb/ft3) 124.0 1230 122.0 121.0 120.0 119.0 118.0 117.0 116.0 115.0 114.0 113.0 1120 Moisture Content (%)

# **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft³): 125 **Corrected Maximum Dry Density** 125

(lb/ft³):

**Optimum Moisture Content (%):** 11 **Corrected Optimum Moisture** 11

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 40; PI = 25

Percent Retained on #4 Sieve = 3.2%; Percent Passing #200 Sieve = 66.4%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S18 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

#### Sample Details

Sample ID: W13-000494-S18 Date Sampled: 3/12/2013

Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (9) Specification: For Informational Purposes Only

Location: LSS-18; 1'-10' Tested By: **Thomas Wagner** 

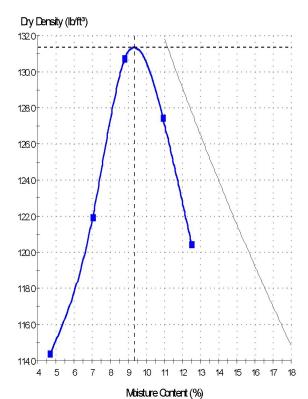
Alternate Sample ID: LSS-18; 1'-10' **Date Submitted:** 3/14/2013

Sampling Method: Soil Boring Auger

**Date Tested:** 3/21/2013



0% Air Voids



#### **Test Results**

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 131 Corrected Maximum Dry Density 131

(lb/ft³):

**Optimum Moisture Content (%):** 9 **Corrected Optimum Moisture** 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 36; PI = 22

Percent Retained on #4 Sieve = 6.4%; Percent Passing #200 Sieve = 58.4%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass Several Routes, NW of Williston

Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S19 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

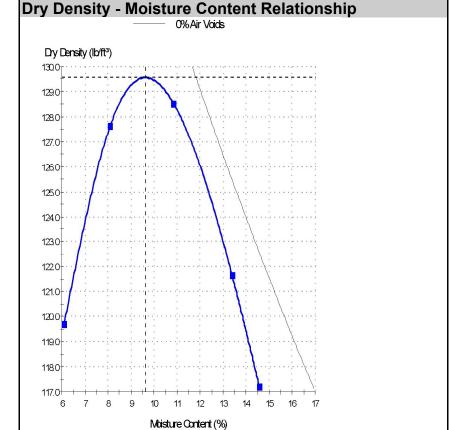
Sample ID: W13-000494-S19

Date Sampled: 3/12/2013 Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (7) Specification: For Informational Purposes Only

Location: LSS-019; 1'-10'

Tested By: Kevin Ficek **Date Tested:** 3/21/2013



#### **Test Results**

3/14/2013

Soil Boring Auger

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 130 **Corrected Maximum Dry Density** 130

(lb/ft³):

Alternate Sample ID: LSS-019; 1'-10'

**Date Submitted:** 

Sampling Method:

**Optimum Moisture Content (%):** 10 **Corrected Optimum Moisture** 10

Content (%):

Method: D

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 34; PI = 20

Percent Retained on #4 Sieve = 12.8%; Percent Passing #200 Sieve = 54.7%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza Fargo, ND, 58102

**Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S20 Issue No: 1

Brianne Nauman

124

EIT

Date of Issue: 4/5/2013

#### Sample Details

Sample ID:W13-000494-S20Alternate Sample ID: LSS-20; 1'-6'Date Sampled:3/12/2013Date Submitted: 3/14/2013Sampled By:John BrooksSampling Method: Soil Boring Auger

Source: Williston NW Bypass Subgrade

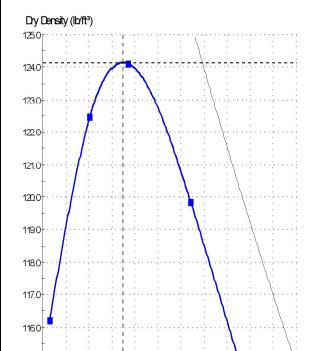
Material: Lean Clay with Sand (CL); A-6 (14)

Specification: For Informational Purposes Only

Location: LSS-20; 1'-6'
Tested By: Shane Falwey

**Date Tested:** 3/22/2013

# Dry Density - Moisture Content Relationship OMAir Voids



11 12 13 14 **Mbisture Content (%)** 

#### **Test Results**

\_\_\_\_\_ AASHTO T 180 - 01 \_\_\_\_ Maximum Dry Density (lb/ft³):

Corrected Maximum Dry Density 124

(lb/ft³):

Optimum Moisture Content (%): 10
Corrected Optimum Moisture 10

Content (%):

Method: A

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

115.0

Assumed Specific Gravity = 2.75

LL = 38; PI = 22

Percent Retained on #4 Sieve = 1.3%; Percent Passing #200 Sieve = 73.6%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S21 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

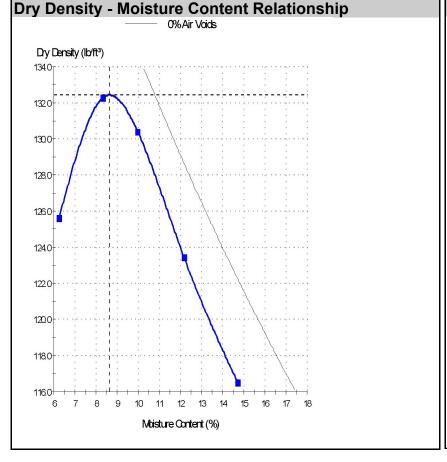
Sample ID: W13-000494-S21 Date Sampled: 3/12/2013

Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (5) Specification: For Informational Purposes Only

Location: LSS-21; 0.5'-6' Tested By:

**Thomas Wagner Date Tested:** 3/21/2013



#### **Test Results**

3/14/2013

Soil Boring Auger

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 132 Corrected Maximum Dry Density 132

(lb/ft³):

Alternate Sample ID: LSS-21; 0.5'-6'

**Date Submitted:** 

Sampling Method:

**Optimum Moisture Content (%):** 9 **Corrected Optimum Moisture** 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 25; PI = 15

Percent Retained on #4 Sieve = 3.2%; Percent Passing #200 Sieve = 54.8%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S22 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

Sample ID: W13-000494-S22

Date Sampled: 3/12/2013 Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-6 (14)

Specification: For Informational Purposes Only Location: LSS-22; 3'-15'

Tested By: **Thomas Wagner**  Alternate Sample ID: LSS-22; 3'-15'

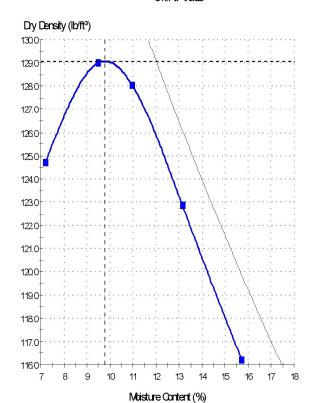
**Date Submitted:** 3/14/2013

**Date Tested:** 

Sampling Method: Soil Boring Auger

**Dry Density - Moisture Content Relationship** 

0% Air Voids



**Test Results** 

3/21/2013

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 129

**Corrected Maximum Dry Density** 129

(lb/ft³):

**Optimum Moisture Content (%):** 10 10

**Corrected Optimum Moisture** Content (%):

Method:

Material on 19.0mm Sieve: Removed Hand round

Rammer Type: Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 39; PI = 27

Percent Retained on #4 Sieve = 3.4%; Percent Passing #200 = 63.0%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S23 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

Sample ID: W13-000494-S23

Date Sampled: 3/12/2013

Sampled By: John Brooks

Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-7-6 (19) Specification: For Informational Purposes Only

Location: LSS-23; 0.5'-15'

Tested By: Shane Falwey Alternate Sample ID: LSS-23; 0.5'-15'

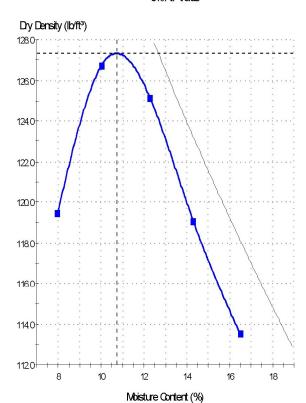
**Date Submitted:** 3/14/2013

**Date Tested:** 

Sampling Method: Soil Boring Auger

**Dry Density - Moisture Content Relationship** 

0% Air Voids



**Test Results** 

3/21/2013

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 127

Corrected Maximum Dry Density 127

(lb/ft³):

**Optimum Moisture Content (%):** 11

**Corrected Optimum Moisture** 

Content (%):

Method:

11

Material on 19.0mm Sieve: Removed

Rammer Type: Hand round

Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 44; PI = 31

Percent Retained on #4 Sieve = 3.6%; Percent Passing #200 Sieve = 69.3%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S24 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/5/2013

Sample Details

Sample ID: W13-000494-S24

Date Sampled: 3/12/2013 Sampled By: John Brooks

Source: Williston NW Bypass Subgrade Material: Clayey Sand (SC); A-6 (3) Specification: For Informational Purposes Only

Location: LSS-24; 2'-7'

Tested By: Shane Falwey Alternate Sample ID: LSS-24; 2'-7'

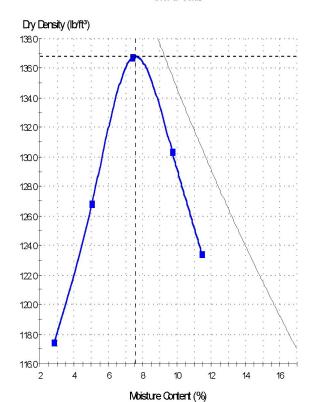
**Date Submitted:** 3/14/2013

**Date Tested:** 

Sampling Method: Soil Boring Auger

**Dry Density - Moisture Content Relationship** 

0% Air Voids



**Test Results** 

3/25/2013

AASHTO T 180 - 01

Maximum Dry Density (lb/ft³): 137 Corrected Maximum Dry Density 137

(lb/ft³):

**Optimum Moisture Content (%):** 8 **Corrected Optimum Moisture** 8

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 26; PI = 13

Percent Retained on #4 Sieve = 5.3%; Percent Passing #200 Sieve = 48.0%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S25

Brianne Nauman

EIT

Date of Issue: 4/5/2013

Sample Details

**Sample ID:** W13-000494-S25

Date Sampled: 3/12/2013 Sampled By: John Brooks

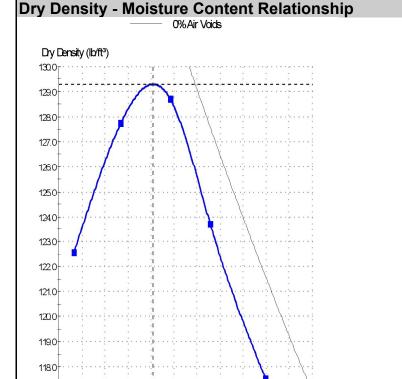
Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (14)

Specification: For Informational Purposes Only

**Location:** LSS-25; 0.5'-10'

Tested By: Kevin Ficek Date Tested: 3/26/2013



#### Test Results

3/14/2013

Soil Boring Auger

AASHTO T 180 - 01

Maximum Dry Density (lb/ft³): 129 Corrected Maximum Dry Density 129

(lb/ft³):

Alternate Sample ID: LSS-25; 0.5'-10'

**Date Submitted:** 

Sampling Method:

Optimum Moisture Content (%): 10
Corrected Optimum Moisture 10

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

117.0

Assumed Specific Gravity = 2.75

LL = 40; PI = 27

Percent Retained on #4 Sieve = 4.3%; Percent Passing #200 Sieve = 64.4%

Moisture Content (%)



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S26 Issue No: 1

Brianne Nauman

EIT

Date of Issue: 4/5/2013

Sample Details

**Sample ID:** W13-000494-S26

Date Sampled: 3/12/2013 Sampled By: John Brooks

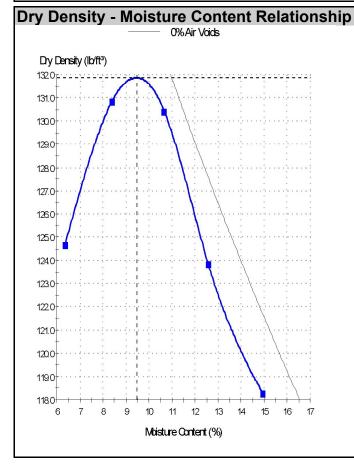
Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (8)

Specification: For Informational Purposes Only

**Location:** LSS-26; 0.5'-10'

Tested By: Kevin Ficek Date Tested: 3/23/2013



#### Test Results

3/15/2013

Soil Boring Auger

AASHTO T 180 - 01 -

Maximum Dry Density (lb/ft³): 132 Corrected Maximum Dry Density 132

(lb/ft³):

Alternate Sample ID: LSS-26; 0.5'-10'

**Date Submitted:** 

Sampling Method:

Optimum Moisture Content (%): 9
Corrected Optimum Moisture 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 30; PI = 19

Percent Retained on #4 Sieve = 2.7%; Percent Passing #200 Sieve = 60.7%



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# **Proctor Report**

Client: Eric Bach

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Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S27

Brianne Nauman

EIT

Date of Issue: 4/9/2013

#### Sample Details

Sample ID: W13-000494-S27

Date Sampled: 3/23/2013

Sampled By: Christopher Elliot

Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (10)

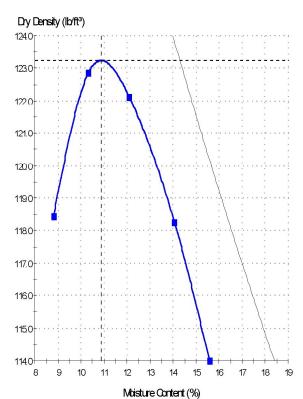
Specification: For Informational Purposes Only

Location: LSS-027; 0.3'-5' **Tested By:** Josh Beringer

Alternate Sample ID: LSS-027; 0.3'-5'
Date Submitted: 3/23/2013
Sampling Method: Soil Boring Auger

**Date Tested:** 4/8/2013





#### Test Results

\_\_\_\_\_\_ AASHTO T 180 - 01 \_ Maximum Dry Density (lb/ft³):

Maximum Dry Density (lb/ft³): 123 Corrected Maximum Dry Density 123

(lb/ft³):

Optimum Moisture Content (%): 11
Corrected Optimum Moisture 11

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round
Visual Description: Dark Brown

#### Comments

Assumed Specific Gravity = 2.75

LL = 37, PI = 21

Percent Retained on #4 Sieve = 4.6%, Percent Passing #200 Sieve = 59.6%



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# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S28 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/9/2013

#### Sample Details

Sample ID: W13-000494-S28 Date Sampled: 3/23/2013 Sampled By: Christopher Elliot

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-7-6 (17) Specification: For Informational Purposes Only

Location: LSS-028; 0.3'-10' Tested By: Josh Beringer

Alternate Sample ID: LSS-028; 0.3'-10'

**Date Submitted:** 3/23/2013

Sampling Method: Soil Boring Auger

**Date Tested:** 4/8/2013

> **Test Results** AASHTO T 180 - 01

> > Maximum Dry Density (lb/ft³): 122 **Corrected Maximum Dry Density** 122

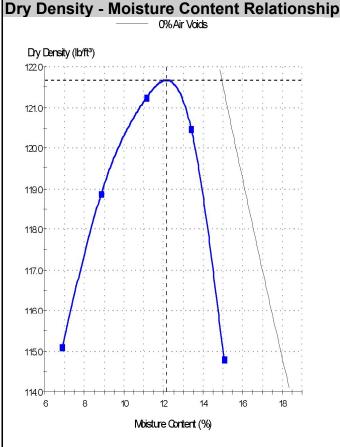
(lb/ft³):

**Optimum Moisture Content (%):** 12 **Corrected Optimum Moisture** 12

Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown



#### Comments

Assumed Specific Gravity = 2.75

LL = 44; PI = 30

Percent Retained on #4 Sieve = 4.2%; Percent Passing #200 Sieve = 67.3%



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Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com Report No: PTR:W13-000494-S29 Issue No: 1

Brianne Nauman

**EIT** 

Date of Issue: 4/9/2013

Sample Details

Sample ID: W13-000494-S29

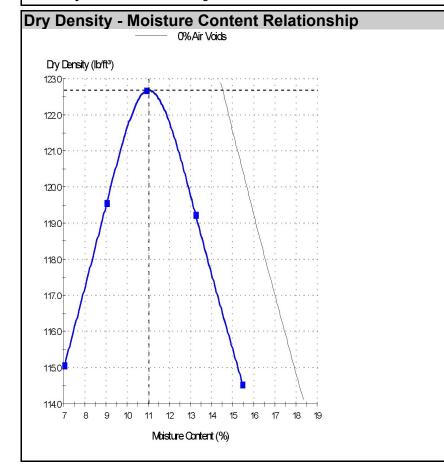
Date Sampled: 3/23/2013

Sampled By: Christopher Elliot

Source: Williston NW Bypass Subgrade Material: Sandy Lean Clay (CL); A-7-6 (16) Specification: For Informational Purposes Only

Location: LSS-029; 0.3'-15'

Tested By: Josh Beringer **Date Tested:** 4/5/2013



#### **Test Results**

3/23/2013

Soil Boring Auger

AASHTO T 180 - 01

Maximum Dry Density (lb/ft3): 123 Corrected Maximum Dry Density 123

(lb/ft³):

Alternate Sample ID: LSS-029; 0.3'-15'

**Date Submitted:** 

Sampling Method:

**Optimum Moisture Content (%):** 11 11

**Corrected Optimum Moisture** Content (%):

Method:

Material on 19.0mm Sieve: Removed Rammer Type: Hand round Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 41; PI = 27

Percent Retained on #4 Sieve = 1.1%; Percent Passing #200 Sieve = 69.2%



1341 South 20th Street, Suite 5, P.O. Box 1836

Bismarck, ND 58504 Phone: 701.255.7180

# **Proctor Report**

Client: Eric Bach

SRF Consulting Group, Inc.

Case Plaza

Fargo, ND, 58102 **Project:** FA-12-00359

Williston NW Bypass

Several Routes, NW of Williston Williams County, ND, 58801

PM: Ezra Ballinger, eballinger@BraunIntertec.com

Report No: PTR:W13-000494-S30 Issue No: 1

Brianne Nauman

EIT

Date of Issue: 4/9/2013

Sample Details

 Sample ID:
 W13-000494-S30

 Date Sampled:
 3/23/2013

Sampled By: Christopher Elliot

Source: Williston NW Bypass Subgrade

Material: Sandy Lean Clay (CL); A-6 (9)

Specification: For Informational Purposes Only

Location: LSS-030; 0.3'-5'
Tested By: Josh Beringer

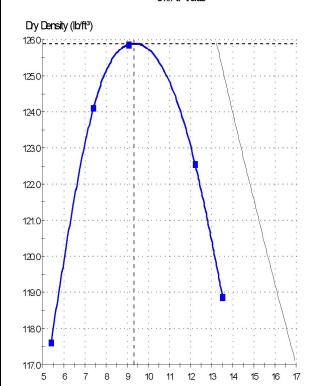
Alternate Sample ID: LSS-030; 0.3'-5'

Date Submitted: 3/23/2013
Sampling Method: Soil Boring Auger

**Date Tested:** 

Dry Density - Moisture Content Relationship

0% Air Voids



**Test Results** 

4/4/2013

\_\_\_\_\_ AASHTO T 180 - 01

Maximum Dry Density (lb/ft³): 126 Corrected Maximum Dry Density 126

(lb/ft³):

Optimum Moisture Content (%): 9
Corrected Optimum Moisture 9

Content (%):

Method:

Material on 19.0mm Sieve: Removed
Rammer Type: Hand round

Visual Description: Brown

Comments

Assumed Specific Gravity = 2.75

LL = 34; PI = 21

Percent Retained on #4 Sieve = 2.5%; Percent Passing #200 Sieve = 60.4%

Moisture Content (%)